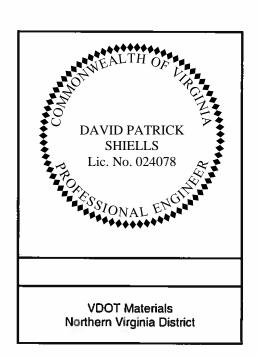


DEPARTMENT OF TRANSPORTATION

DIVISION: MATERIALS

REPORT COVER SHEET

Pavement Investigation and Evaluation
for Town of Waterford
September 16, 2016
 David P. Shiells, P.E.



Responsible for All Pages

Project Description Town of Waterford

From: N/A
To: N/A
Project UPC No.: 100418



DEPARTMENT OF TRANSPORTATION

4975 Alliance Drive Fairfax, VA 22030

CHARLES A. KILPATRICK, P.E. COMMISSIONER

September 16, 2016

MEMORANDUM

TO:

Mr. Michael Gleasman, P.E.

SUBJECT: Pavement Investigation in the Town of Waterford

Loudoun County, Virginia

As requested, we have completed a review of the condition of the existing pavements within the Town of Waterford. As described to us, the main concerns for the roadways within the town are to provide the most effective short-term pavement resurfacing and to provide for long-term pavement performance. Local residents are also interested in lowering the existing pavement surface at specific locations in order to mitigate surface water flow into their residences and businesses and to allow for more convenient street parking.

The Town of Waterford lies mainly on the side of a large hill, occupying the area between Catoctin Creek and Clarkes Gap Road. Numerous historic structures including residences and businesses are scattered throughout the town among more recently constructed buildings and dwellings. Many of the structures are separated from the existing streets by narrow sidewalks, boardwalks, shallow ditches, or ditches which have been infilled over time. There are three main thoroughfare roadways and three connecting streets within the town limits. The three thoroughfares are identified as the north-south stretch of pavement of Route 665 variously named Clarkes Gap Road/High Street/ Butchers Row/Loyalty Road; the southeasterlynorthwesterly stretch of pavement of Route 662 variously named Factory Street/Second Street/West Main Street/First Street and the easterly-westerly pavement of Route 698 named Water Street. The three connecting streets all are oriented east-west and named East Main Street, Patrick Street and Janney Street.

The provision of conveyances for surface drainage throughout the town is very poor to nonexistent. This is likely due to the lack of adequate ditches, buried or plugged driveway pipes, and inadequate storm water facilities. As a result, streets within the town have become the paths of least resistance for precipitation to reach Catoctin Creek. Numerous town residents and business owners have begun to construct sandbag barriers in order to mitigate the flow of water from the streets into their buildings. Gravel washouts onto the paved streets, from adjacent driveways, shoulders, and alleys, are also common.

Existing Pavement Conditions

The typical sections within the Town of Waterford consist of a variety of very old, non-standard curb and gutter and open shoulder configurations. The width of the pavement varies from approximately 14' on Patrick Street to approximately 21' on High Street near the intersection with Patrick Street. On-street parking on many of the town's thoroughfares, particularly on West Main Street, reduces the effective travel width to just one lane, forcing oncoming traffic to yield wherever pull off openings are available. The pavement is old and its condition ranges from fair to very poor, with occasional longitudinal cracking, moderate to severe alligator cracking, moderate rutting, open longitudinal joints, pavement edge failures, and potholes. Some of the pavement surfaces appear to be very old, particularly along the lesser-used streets (East Main Street and Patrick Street). The only apparent utility beneath the pavements appears to be a sanitary sewer system. However, visually, the existing pavement distress does not consistently follow the path of the sanitary sewer beneath the pavement. Figures 1 through 7 illustrate the variable existing pavement conditions.

Where it exists, most of the curb and gutter has been either buried beneath repeated asphalt pavement overlays and/or destroyed by traffic abuse or subgrade movement. Visually, it appears that repeated overlays and build-up of the pavement have created conditions in a few locations where the existing pavement surface is significantly higher than the adjacent sidewalk/curb and gutter. This is particularly true along the west side of Second Street near its intersection with West Main Street and along the south side of West Main Street. Figures 8 through 10 illustrate the existing curb and gutter and pavement build-up conditions.

Existing Drainage Conditions

There are very few deep or continuous stretches of ditch within Waterford. From the occasional appearance of driveway cross-pipes, many of which are partially or completely buried, it appears that ditching along both sides of the pavement previously existed. However, many of those ditches have been infilled and the space is now used for near-street parking. Anecdotally, townspeople told us that a group of concerned citizens has begun the process of marking the buried driveway pipes in hopes of having VDOT clear them. However, restoration of the ditches between the driveways is not an issue that the residents appear ready to discuss, likely due to the potential loss of parking space. Figures 11 through 16 illustrate the inadequate or nonexistent pavement drainage.

Where runoff encounters high spots along the pavement edge, due to the slight crown in the pavement, puddles form. The resultant standing water adjacent to the pavement edge allows

runoff to inundate the pavement and granular subbase, exposing the pavement section to extended periods of saturation. Potholes and edge failures are coincident with evidence of ponded water along the pavement edge throughout the town.

Field Investigation

A total of 28 locations were cored and drilled to a maximum depth of 6.7 feet below the existing pavement surface in order to determine the structure and condition of the existing pavement and to investigate the subgrade soils. The borings were performed at the following locations:

- Factory Street: CB-1, CB-2
- Second Street: C-3A, C-3B, CB-4, CB-5, CB-6A, CB-6B, C-7A, C-7B
- West Main Street and First Street: C-8A, CB-8B, C-9, C-10, CB-11, CB-12
- East Main Street: CB-13A, CB-13B, CB-14
- Patrick Street: CB-15, CB-16
- Water Street: CB-17, C-18
- Clarkes Gap Road/High Street/Butchers Row/Loyalty Road: CB-19A, CB-19B, CB-20, C-21A, CB-21B

Cores were not obtained from Janney Street due to the very poor condition of the existing surface treatment (tar and chip) where most of the surface treatment has reverted back to unbound aggregate. Cores were also not obtained from Church Street since it appears to be only gravel surfaced and locally treated as a private alleyway.

Detailed descriptions and photographs of the asphalt layers encountered are included on the attached boring logs, and are shown on the attached boring location photos. Boring location plans are also attached.

Asphalt Pavement Structure, Subbase and Subgrade Soil Conditions

The existing pavement structure is quite variable, both in thickness and condition, ranging from 8" thick along the western end of Factory Street to less than 2" thick on East Main Street near the intersection with Second Street. The condition ranges from moderately stripped and oxidized to completely stripped and friable. Specific to each street, the asphalt pavement thicknesses measured in the cores were:

- Factory Street: 5.0" to 8.0" (average 6.5")
- <u>Second Street</u>: 3.6" to 7.5" (average 5.7")
- West Main Street and First Street: 4.1" to 7.5" (average 6.1")
- East Main Street: 1.9" to 6.0" (average 4.6")
- Patrick Street: 4.1" to 6.0" (average 5.1")
- <u>Water Street</u>: 3.1" to 5.0" (average 4.1")
- Clarkes Gap Road/High Street/Butchers Row/Loyalty Road: 3.8" to 7.1" (average 5.2")

Most of the streets contain some amount of both open and patched potholes. The patched areas appear to have been repaired during multiple visits, over a long period of time, with materials

ranging from hot-mixed asphalt, to cold-patch asphalt, to sprayed temporary asphalt repair material.

The substantially raised pavement profile, adjacent to businesses and residences along Second Street and West Main Street, does not appear to be entirely due to repeated overlay build up. Cores obtained at C-7A and C-7B along Second Street indicate only an average asphalt thickness of 6.6". However, the existing surface of Second Street is in excess of 15" above the adjacent sidewalk. On West Main Street, at core locations C-9 and C-10 where the most substantial raised profile exists, an average asphalt thickness of 6.7" was measured. The adjacent sidewalk along the south side of West Main Street varies between 7" and 10" below the elevation of the pavement. As a result, it appears that the existing, raised pavement elevation is due to, or in addition to, factors other than repeated overlay/build-up of the pavement. Anecdotally, townspeople noted to us that they believed the profile was intentionally raised to accommodate more convenient geometric conditions on Second Street and West Main Street. Apparently, there was quite a history of difficult turns due to elevation differences in adjacent roadway pavement surfaces, particularly at the intersection Main, Second and Water Streets.

Granular subbase beneath the asphalt pavement consists mainly of a coarse soil fill comprised of SILTY F-C SAND with f-c gravel or CLAYEY F-C SAND AND F-C GRAVEL. Quarried, crushed aggregate subbase was only encountered in borings C-7A and C-8A and may be associated with episodes of either utility or pavement repair. The granular subbase ranged from 2.4" thick in CB-14 on East Main Street to greater than 13" of crushed aggregate in C-7A on Second Street immediately south of the West Main Street intersection.

The subgrade generally consists of LEAN CLAYS, SILTY and CLAYEY SANDs and SANDY SILTS, all with varying amounts of gravel. The soils occur as both man-made FILL and residuum of the parent gneiss found at depth at this location. In general, the soils range from medium dense/very stiff to loose/firm with average SPT (uncorrected) "N" values of between 5 and 25 blows per foot. Overall, the subgrade soils are moist, with in-situ moisture contents averaging between 15% and 20%. Optimum moisture contents of the four subgrade samples that were tested ranged from 11.8% to 21.2% (average of 14.5%).

No evidence of ground water was encountered during drilling in any of the borings. The borings were backfilled and patched immediately after completion. Detailed descriptions and photographs of the asphalt layers encountered are included on the boring logs.

Conclusions

The streets within the Town of Waterford are paved with old to very old asphalt which is in fair to very poor condition. Evidence of the distress in the pavement is obvious in many locations as indicated by the existence of longitudinal cracking, moderate to severe alligator cracking, moderate rutting, open longitudinal joints, pavement edge failures, and potholes. The almost complete lack of surface drainage adjacent to the pavement, likely a significantly contributing factor to the various pavement failures, has caused town residents to adopt a defensive position toward the roadways, as the streets have become the defacto surface drainage system throughout the town. In addition to the lack of drainage, many of the pavements are structurally inadequate for the current traffic volumes. The severe alligator cracking at various locations and the moderate rutting on East Main Street and Patrick Street immediately west of High Street are indicative of structural pavement failure over subgrades which are bearing traffic loads beyond their capacity. Also, many of the cores have moderate to nearly complete stripping of the bituminous binder, particularly in the layers beneath the most recent overlay, compromising the structural integrity of the pavement.

Recommendations

Surface and Pavement Drainage

As noted previously, there is a substantial lack of conveyances for water to drain off of, and away from, the roadways throughout Waterford. Any plan for restoration of the pavements in Waterford must include a strategy for provision of adequate facilities to allow the pavement to drain. These should include re-establishment of pavement edge ditches; exposure, unplugging, and/or replacement of driveway pipes; and connection of the ditches to existing (and perhaps newly constructed) facilities to allow water to reach Catoctin Creek. Failure to improve the drainage throughout the town will reduce the expected pavement life.

Pavement Restoration

The following options are presented in order of least risk/highest cost/longest potential lifespan to greatest risk/lowest cost/shortest potential lifespan and are broken down into the five streets described as Route 662, Route 665, Route 698, East Main Street (Route 785), and Patrick Street (Route 783).

Please note that if reconstruction is chosen on any of the streets, a careful examination of the subgrade conditions, including proof-rolling of the subgrade with a loaded dump truck, will be required. We anticipate that some undercut and replacement of unsuitable subgrade soils (primarily due to high moisture content) will be required. Also note that an advantage of demolition and reconstruction is control of the surface grade. Areas of existing pavement where the surface is substantially above the adjacent sidewalk could easily be lowered if the existing roadway were demolished and reconstructed.

Route 662 (Factory Street/Second Street/West Main Street/First Street)

Based on an analysis of the existing and projected traffic volumes along Route 662, assuming a 20-year secondary pavement design scenario, the existing pavement thickness deficit ranges from adequate (no build up required) to 4.1" (avg. 1.6") required to meet a 20-year design.

Please note that due to the deficit in available pavement section thickness, it will not be possible to *reduce* the existing pavement thickness in order to lower the surface. Reducing the existing pavement section, even if replaced with a shallower thickness of *new* pavement, will reduce the potential lifespan of the pavement and lead to premature failure. Only Option 1 will allow changes to the pavement surface elevation.

Option 1 – Complete Reconstruction

To meet a 20-year design life, we recommend complete demolition of the existing pavement and reconstruction with:

Surface: 1" Asphalt Concrete Type SM-4.75A, estimated at 118 lbs/sy

Base: 5" Asphalt Concrete, Type BM-25.0A

Subbase: 6" Aggregate Base Material, Type I, Size no. 21B extended to daylight in adjacent ditches or connected to UD-4 edgedrains beneath adjacent curb and gutter.

Option 2 - Mill and Overlay Buildup

In order to provide a longer-lasting pavement structure without complete reconstruction, it would be advantageous to add to the existing pavement thickness by removing the existing surface and building up the pavement. We recommend that the existing pavement be milled to a depth of 1.5" to remove the existing severely deteriorated and debonded surface. Any severe failures should be patched full depth (4") with BM-25.0A prior to building up the existing pavement with 2" of SM-12.5A and 1" of SM-4.75A. However, given the existing, highly variable pavement conditions and the geometric controls presented by the thin roadway, nearby dwellings/businesses, and adjacent driveways, it is unlikely that a buildup of the pavement will be possible between Patrick Street and Leggett Street. For these areas, we recommend that the existing pavement be milled to a depth of 2.5" to remove the existing severely deteriorated and debonded surface. Any severe failures should be patched full depth (4") with BM-25.0A prior to building up the existing pavement with 1.5" of SM-12.5A and 1" of SM-4.75A.

Option 3 – Mill and Overlay (no net buildup)

In order to improve the existing surface condition—with no improvement in the potential pavement life and no change in the current grade (i.e. an aesthetic and short-term functional improvement)—we recommend the following:

Mill the existing pavement to a depth of 2", patch the severe failures full depth (4") with BM-25.0A and replace with 2" Asphalt Concrete Type SM-9.5A, estimated at 230 lbs/sy

Route 665 (Clarkes Gap Road/High Street/ Butchers Row/Loyalty Road)

Based on an analysis of the existing and projected traffic volumes along Route 665, assuming a 20-year secondary pavement design scenario, the existing pavement thickness deficit ranges from approximately 1.8" to 5.9" (avg. 3.6") required to meet a 20-year design.

<u>Option 1 – Complete Reconstruction</u>

To meet a 20-year design life, we recommend complete demolition of the existing pavement and reconstruction with:

Surface: 1" Asphalt Concrete Type SM-4.75A, estimated at 118 lbs/sy

Base: 5.5" Asphalt Concrete, Type BM-25.0A

Subbase: 6" Aggregate Base Material, Type I, Size no. 21B extended to daylight in adjacent ditches or connected to UD-4 edgedrains beneath adjacent curb and gutter.

Option 2 - Mill and Overlay Buildup

In order to afford a longer-lasting pavement structure without complete reconstruction, it would be advantageous to add to the existing pavement thickness by removing the existing surface and building up the pavement. Given that the roadway is primarily a thoroughfare with few private or commercial entrances, a buildup of the existing pavement may be more appropriate than on Route 662. We recommend that the existing pavement be milled to a depth of 2" to remove the existing severely deteriorated surface. Any severe failures should be patched full depth (6") with BM-25.0A prior to building up the existing pavement with 4.5" of BM-25.0A and 1" of SM-4.75A. This will produce a net buildup of 3.5" and meet the thickness requirement for a 20-year design life.

Option 3 – Mill and Overlay (no net buildup)

In order to improve the existing surface condition—with no improvement in the potential pavement life and no change in the current grade (i.e. an aesthetic and short-term functional improvement)—we recommend the following:

Mill the existing pavement to a depth of 2", patch the severe failures full depth (6") with BM-25.0A and replace with 2" Asphalt Concrete Type SM-9.5A, estimated at 230 lbs/sy

Route 698 (Water Street)

Based on an analysis of the existing and projected traffic volumes along Route 698, assuming a 20-year secondary pavement design scenario, the existing pavement thickness deficit ranges from adequate to 1.9" (avg. 0.4") required to meet a 20-year design.

Option 1 - Complete Reconstruction

To meet a 20-year design life, we recommend complete demolition of the existing pavement and reconstruction with:

Surface: 1" Asphalt Concrete Type SM-4.75A, estimated at 118 lbs/sy

Base: 3.5" Asphalt Concrete, Type BM-25.0A

Subbase: 6" Aggregate Base Material, Type I, Size no. 21B extended to daylight in adjacent ditches or connected to UD-4 edgedrains beneath adjacent curb and gutter.

Option 2 - Mill and Overlay Buildup

In order to afford a longer-lasting pavement structure without complete reconstruction, it would be advantageous to add to the existing pavement thickness by removing the existing surface and building up the pavement. Given that the roadway is primarily a thoroughfare with few private or commercial entrances, a buildup of the existing pavement may be appropriate. We recommend that the existing pavement be milled to a depth of 2" to remove the existing severely deteriorated surface. Any severe failures should be patched full depth (4") with BM-25.0A prior to building up the existing pavement with 2" of IM-19.0A and 1" of SM-4.75A. This will produce a net buildup of 1" and meet the thickness requirement for a 20-year design life.

Option 3 - Mill and Overlay (no net buildup)

In order to improve the existing surface condition—with no improvement in the potential pavement life and no change in the current grade (i.e. an aesthetic and short-term functional improvement)—we recommend the following:

Mill 2" of the existing surface, patch the severe failures full depth (4") with BM-25.0A and replace with 2" Asphalt Concrete Type SM-9.5A, estimated at 230 lbs/sy

East Main Street (Route 785)

Based on an analysis of the existing and projected traffic volumes along East Main Street (between Second Street and High Street), assuming a 20-year secondary pavement design scenario, the existing pavement thickness deficit ranges from approximately 0.9" to 3.6" (avg. 2.3") required to meet a 20-year design. However, given the extensive amount of pavement failures along East Main Street, we strongly recommend that *Option 1 - Complete Reconstruction* be considered as the best option.

Option 1 – Complete Reconstruction

To meet a 20-year design life, we recommend complete demolition of the existing pavement and reconstruction with:

Surface: 1" Asphalt Concrete Type SM-4.75A, estimated at 118 lbs/sy

Base: 3.5" Asphalt Concrete, Type BM-25.0A

Subbase: 6" Aggregate Base Material, Type I, Size no. 21B taken to daylight in adjacent

ditches or connected to UD-4 edgedrains beneath adjacent curb & gutter.

Anecdotally, we were told by residents of the town that a group of concerned citizens has petitioned Loudoun County and VDOT for replacement of an existing concrete/rubble ditch which separates East Main Street from a historical parcel—currently a small park—along the north side of East Main Street near the intersection with Water Street. Plans for the ditch reconstruction have apparently been drawn up and submitted for approval. If demolition and

reconstruction are chosen, we recommend that pavement reconstruction be coordinated with the ditch reconstruction project.

Option 2 – Mill and Overlay Buildup

In order to afford a longer-lasting pavement structure without complete reconstruction, it would be advantageous to add to the existing pavement thickness by removing the existing surface and building up the pavement. There are only a few private or commercial entrances along East Main Street, allowing for relatively easy buildup of the existing pavement.

Please note that any milling of the pavement may expose the existing coarse soil subbase, given that the pavement thickness varies considerably between 1.9" and 6" in thickness.

We recommend that the existing pavement be milled to a depth of 2" to remove the existing severely deteriorated surface. Any severe failures should be patched full depth (4") with BM-25.0A prior to building up the existing pavement with 3.5" of BM-25.0A and 1" of SM-4.75A. This will produce a net buildup of 2.5" and meet the thickness requirement for a 20-year design life.

Option 3 - Mill and Overlay (no net buildup)

In order to improve the existing surface condition—with no improvement in the potential pavement life and no change in the current grade (i.e. an aesthetic and short-term functional improvement)—we recommend the following:

Mill 2" of the existing surface, patch the severe failures full depth (6") with BM-25.0A and replace with 2" Asphalt Concrete Type SM-9.5A, estimated at 230 lbs/sy. Please note that any milling of the pavement may expose the existing coarse soil subbase, given that the pavement thickness varies considerably between 1.9" and 6" in thickness.

Patrick Street (Route 783)

Based on an analysis of the existing and projected traffic volumes along Route 698, assuming a 20-year secondary pavement design scenario, the existing pavement thickness deficit ranges from 0.5" to 1.6" (avg. 1.1") required to meet a 20-year design.

Please note that the complete deterioration of the existing concrete curb and gutter along the north side of Patrick Street will necessitate replacement of the curb and gutter along with the pavement for all of the options below.

<u>Option 1 – Complete Reconstruction</u>

To meet a 20-year design life, we recommend complete demolition of the existing pavement and reconstruction with:

Surface: 1" Asphalt Concrete Type SM-4.75A, estimated at 118 lbs/sy

Base: 3.5" Asphalt Concrete, Type BM-25.0A

Subbase: 6" Aggregate Base Material, Type I, Size no. 21B extended to daylight in adjacent ditches or connected to UD-4 edgedrains beneath adjacent curb and gutter.

Option 2 - Mill and Overlay Buildup

In order to afford a longer-lasting pavement structure without complete reconstruction, it would be advantageous to add to the existing pavement thickness by removing the existing surface and building up the pavement. Please note that any milling of the pavement may expose the existing stripped asphalt base.

We recommend that the existing pavement be milled to a depth of 2" to remove the existing severely deteriorated surface. Any severe failures should be patched full depth (4") with BM-25.0A prior to building up the existing pavement with 3" of BM-25.0A and 1" of SM-4.75A. This will produce a net buildup of 2" and meet the thickness requirement for a 20-year design life.

Option 3 - Mill and Overlay (no net buildup)

In order to improve the existing surface condition—with no improvement in the potential pavement life and no change in the current grade (i.e. an aesthetic and short-term functional improvement)—we recommend the following:

Mill 2" of the existing surface, patch the severe failures full depth (4") with BM-25.0A and replace with 2" Asphalt Concrete Type SM-9.5A, estimated at 230 lbs/sy.

Janney Street

If Janney Street is included in the resurfacing plan, we recommend demolition of the existing deteriorated surface treatment and replacement with <u>Option 1</u> as recommended for East Main Street.

If you have any questions, please contact Carlin Hall at (703) 259-2745.

Prepared by:

Carlin L. Hall, II

Asst. District Materials Engineer

For:

David P. Shiells, P.E.

District Materials Engineer

Cc: Mr. Sunil Taori, P.E.

Mr. Harihar Shiwakoti, P.E.

Attachments: Figures 1 through 16 (6)

Boring Location Plans (4)

Core Location Photographs (19)

Boring Logs (28)

Summary of Soils Laboratory Test Data (1)

Soils Laboratory Test Results (21)



Figure 1: West Main Street south of Clover Hill Road



Figure 2: East Main Street between Second Street and High Street



Figure 3: Second Street north of Janney Street



Figure 4: High Street between Patrick Street and Janney Street.



Figure 5: Water Street near its intersection with Butchers Row.



Figure 6: Patrick Street between High Street and Second Street.



Figure 7: Factory Street between High Street and Second Street.



Figure 8: West Main Street west of Second Street. Note the substantially raised pavement elevation adjacent to the sidewalk. The exposed curb consists of pieces of decorative slate/shale, stood on edge.



Figure 9: Second Street south of West Main Street. Note the substantially raised pavement elevation adjacent to the sidewalk. The exposed curb consists of deteriorated hydraulic cement concrete. The gutter pan is completely buried; only the top surface of the curb is exposed.



Figure 10: West Main Street west of Second Street. Note the substantially raised pavement elevation adjacent to the sidewalk. The exposed curb consists of hydraulic cement concrete. The gutter pan is buried by a few inches of asphalt.



Figure 11: Second Street between Janney Street and Patrick Street. The short stakes mark the ends of a completely buried driveway cross-pipe. Note the complete lack of ditches.



Figure 12: Sandbagging in front of 15539 Second Street (south of Patrick Street). Note the lack of a ditch and a vehicle parked in the backfilled ditch line.



Figure 13: Factory Street between High Street and Second Street. A loosely-constructed concrete cap has been poured over the end of the pipe which crosses beneath Factory Street. The end of the pipe was subsequently buried. An adjacent homeowner has dug below the cap to expose the end of the pipe, apparently in an attempt to allow water to drain. There is only a shallow ditch in the foreground and none at the far end of the pipe.



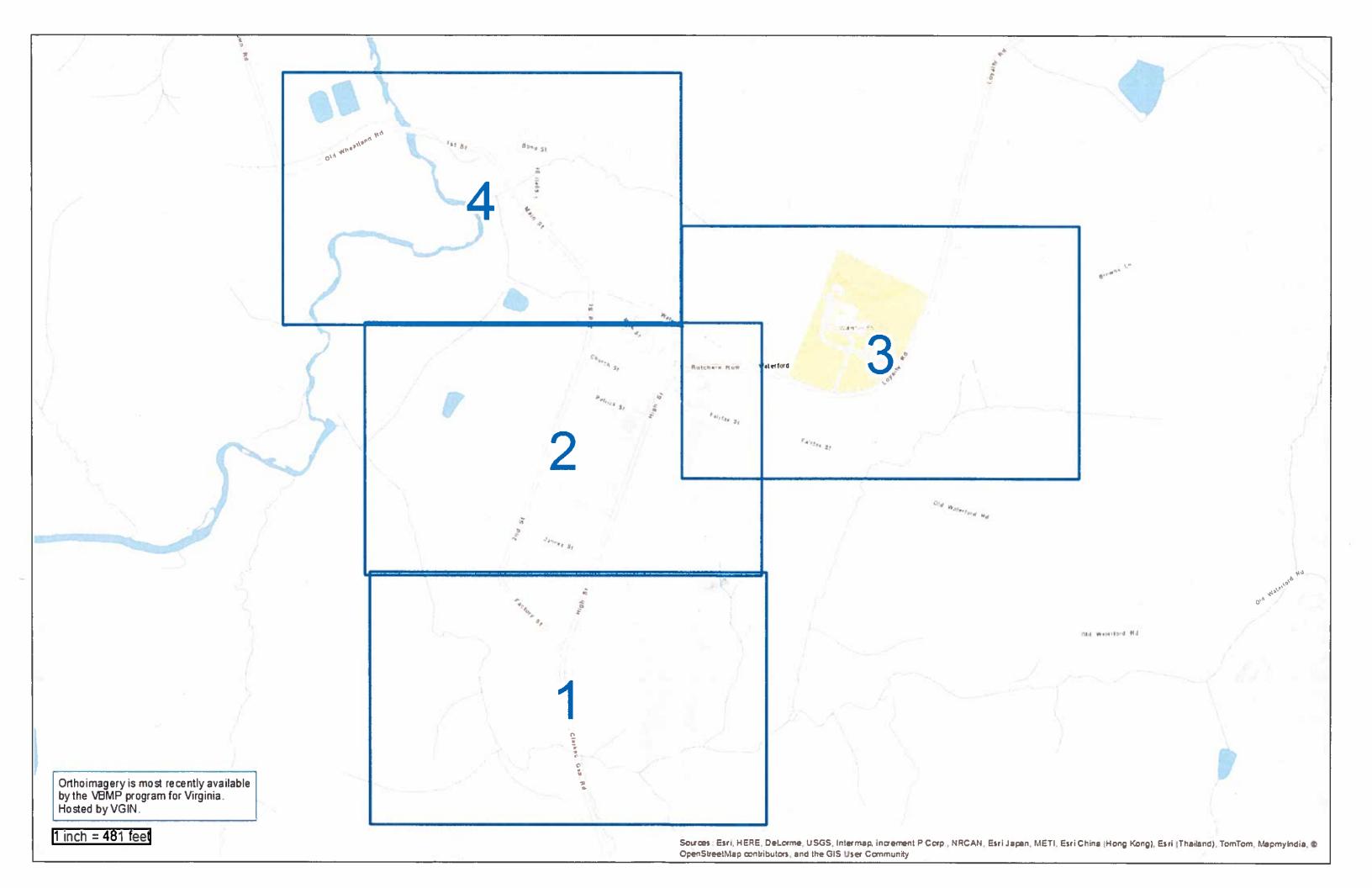
Figure 14: Partially buried pipes in front of 40152 West Main Street. Note the vehicle parked in the partially filled ditch line.

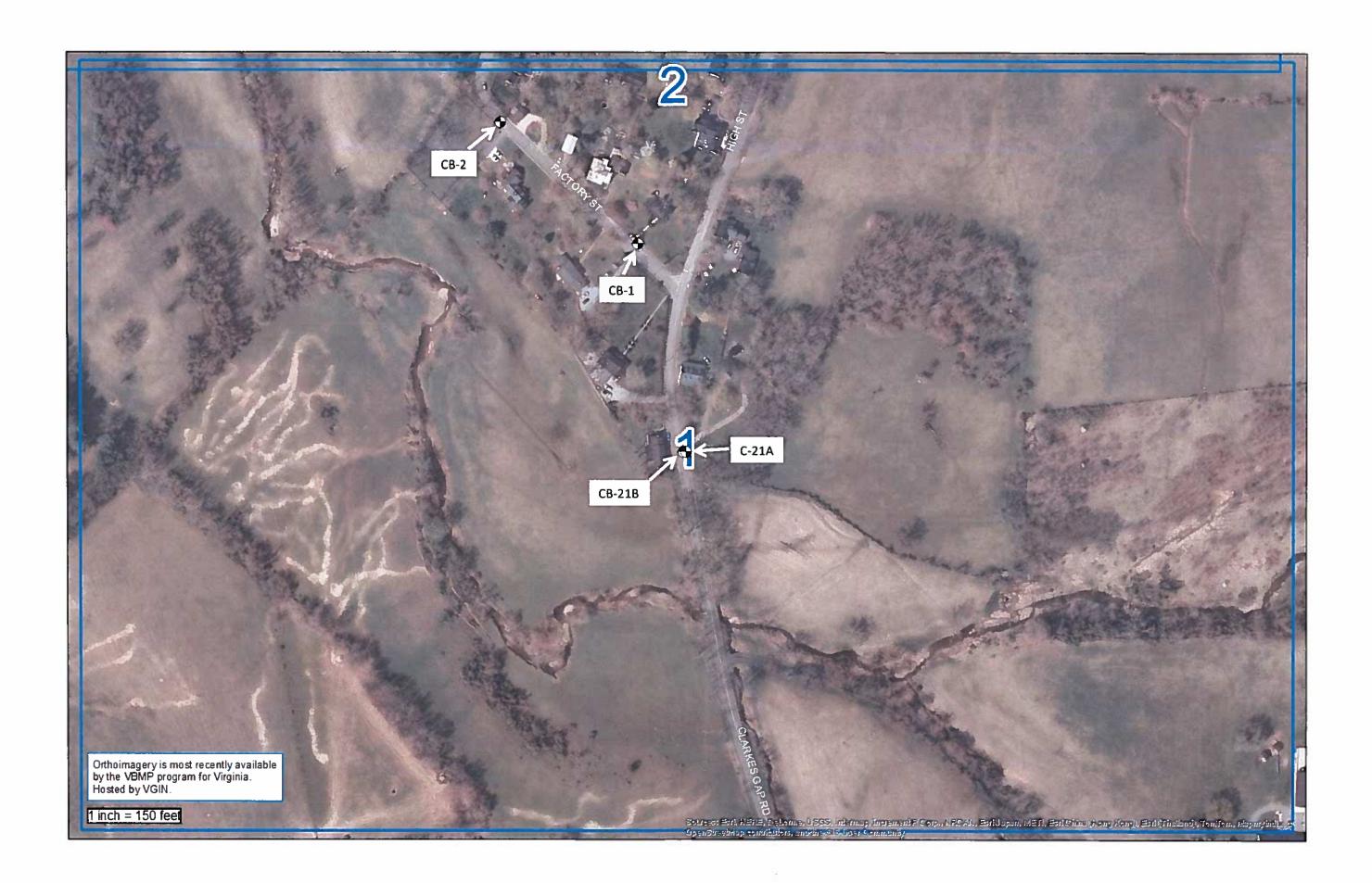


Figure 15: Partially buried/crushed pipe at intersection of Second Street and Factory Street. The pipe is located in the depression to the right at the foot of the crushed warning sign. Water puddles at the low point of the curve (pipe inlet), saturating the pavement. Note the adjacent pavement failures and repairs.



Figure 16: Butchers Row between
East Main Street and Water
Street/Loyalty Road. Note the lack of ditches on either side of the pavement.

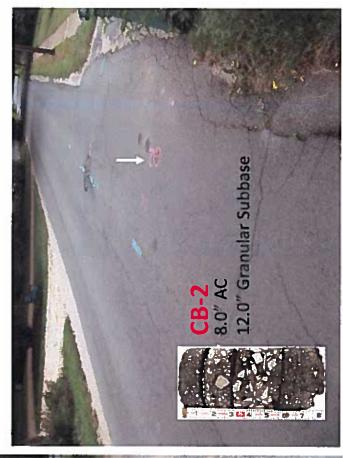




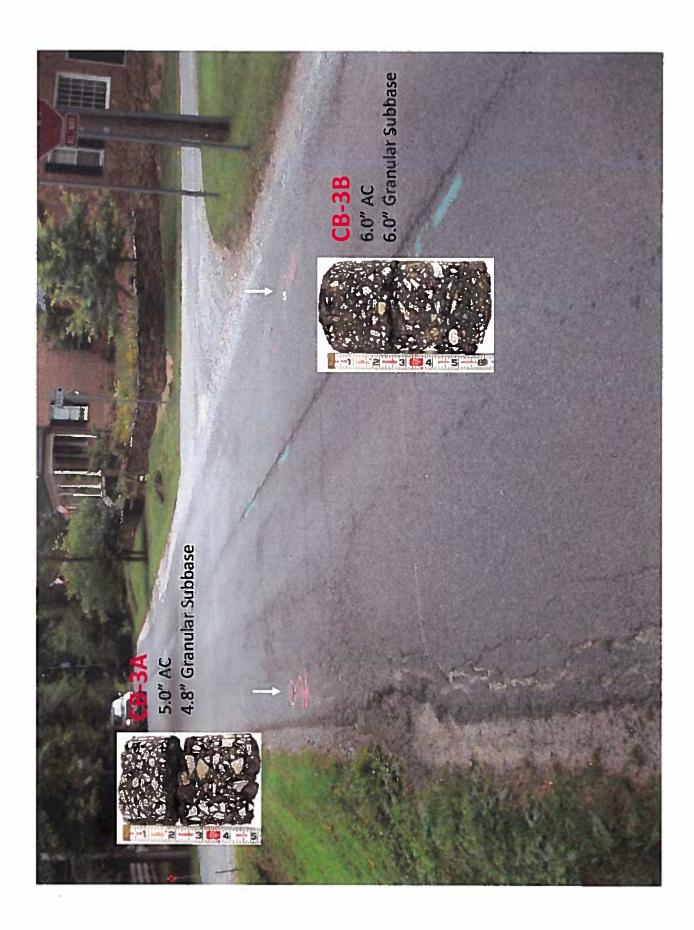




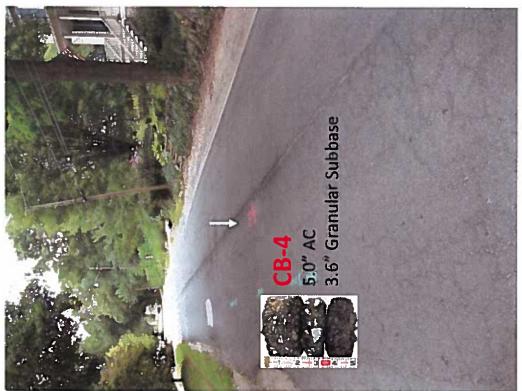


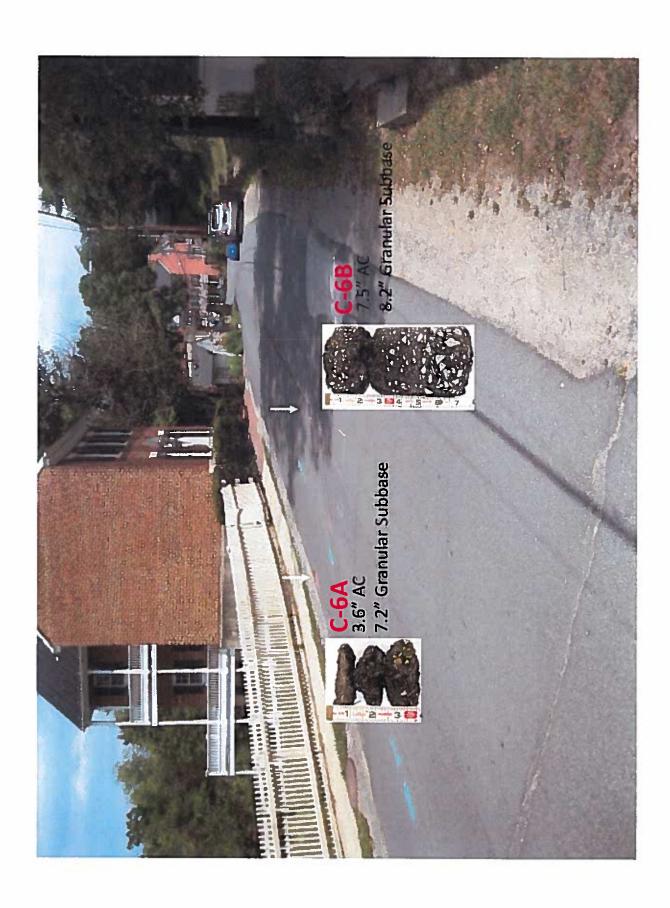








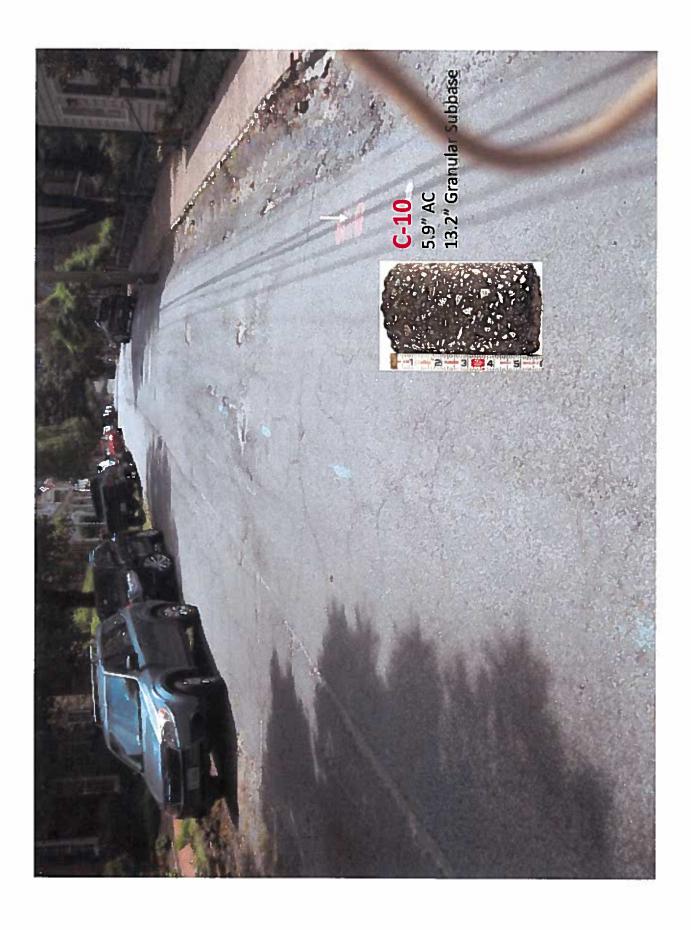


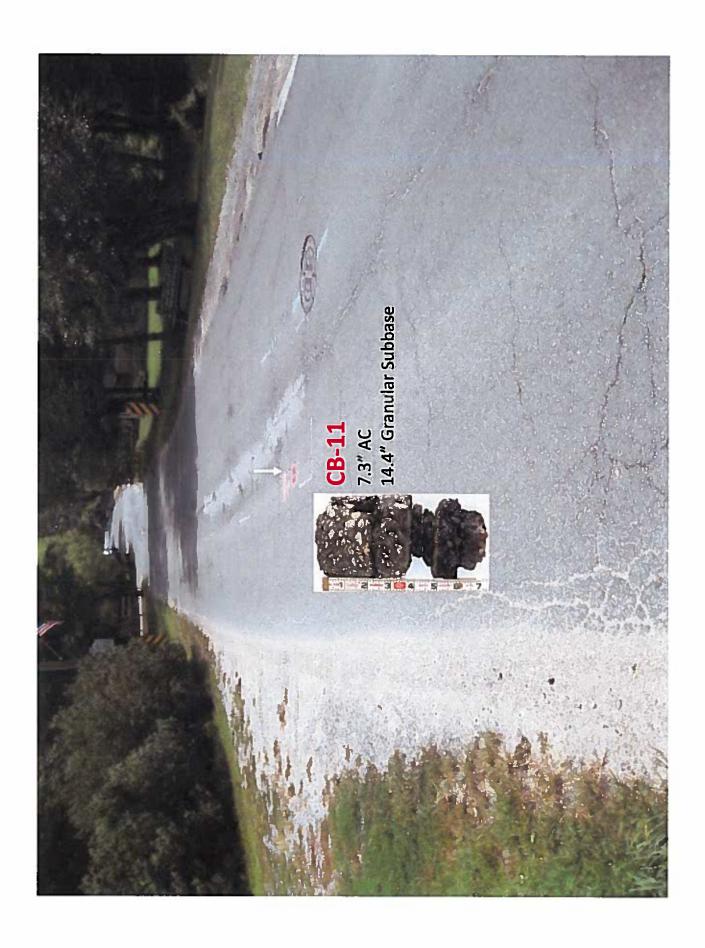


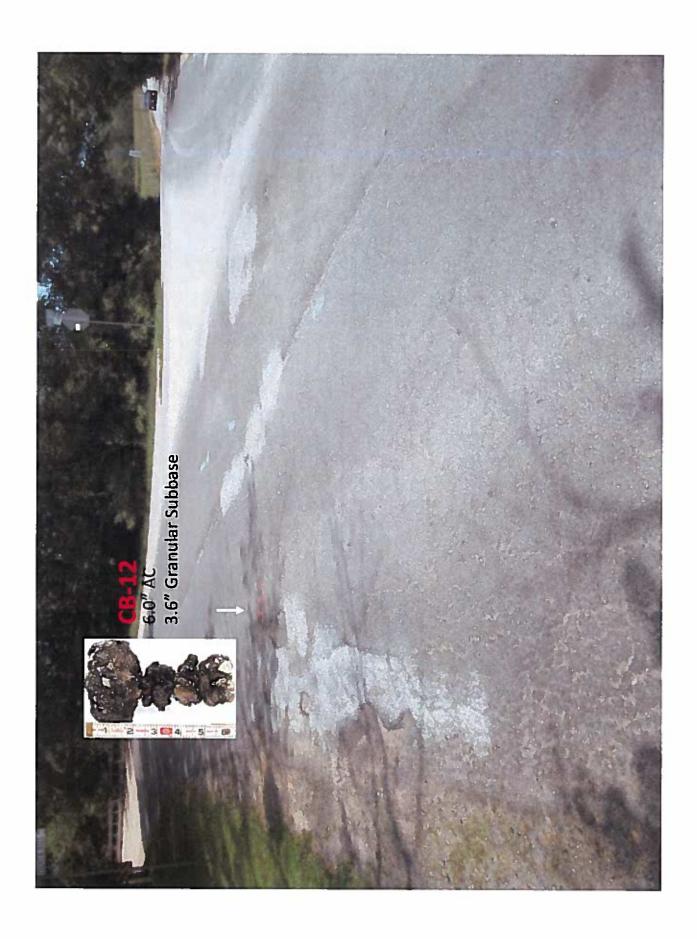


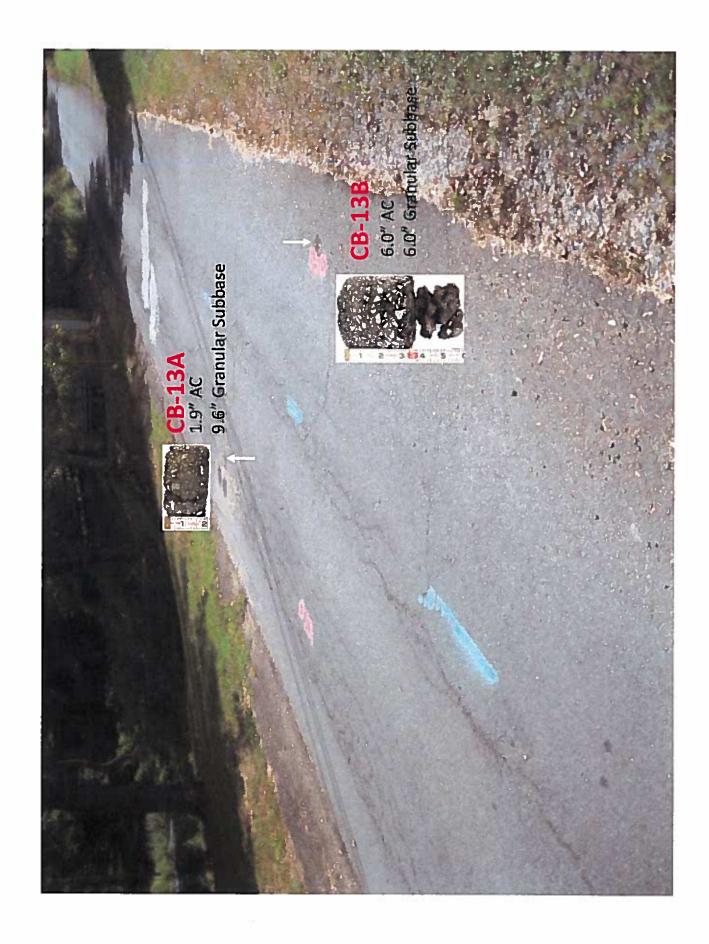


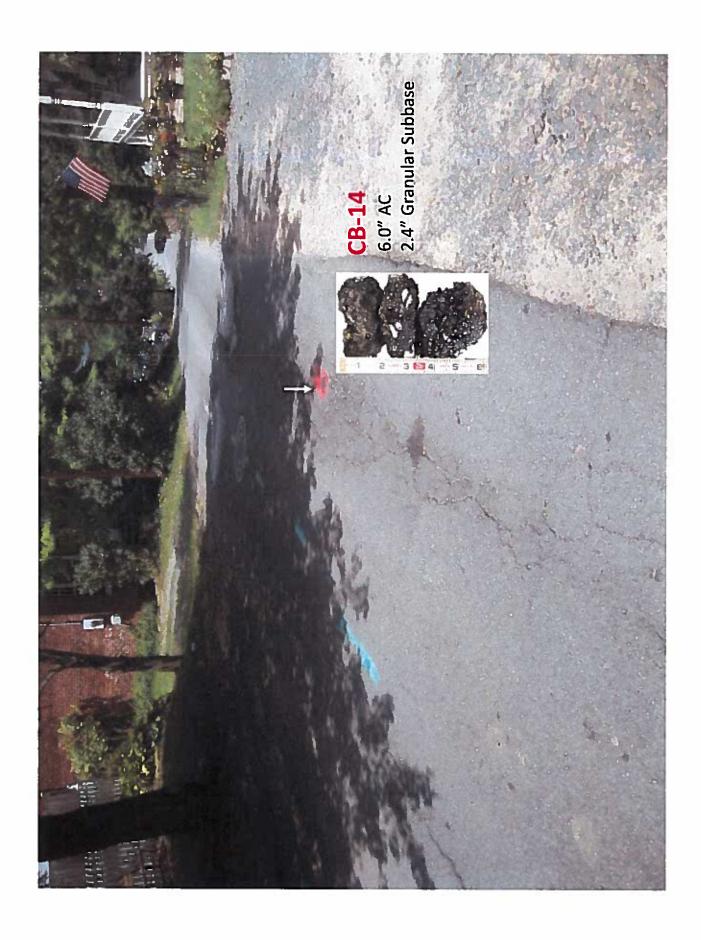


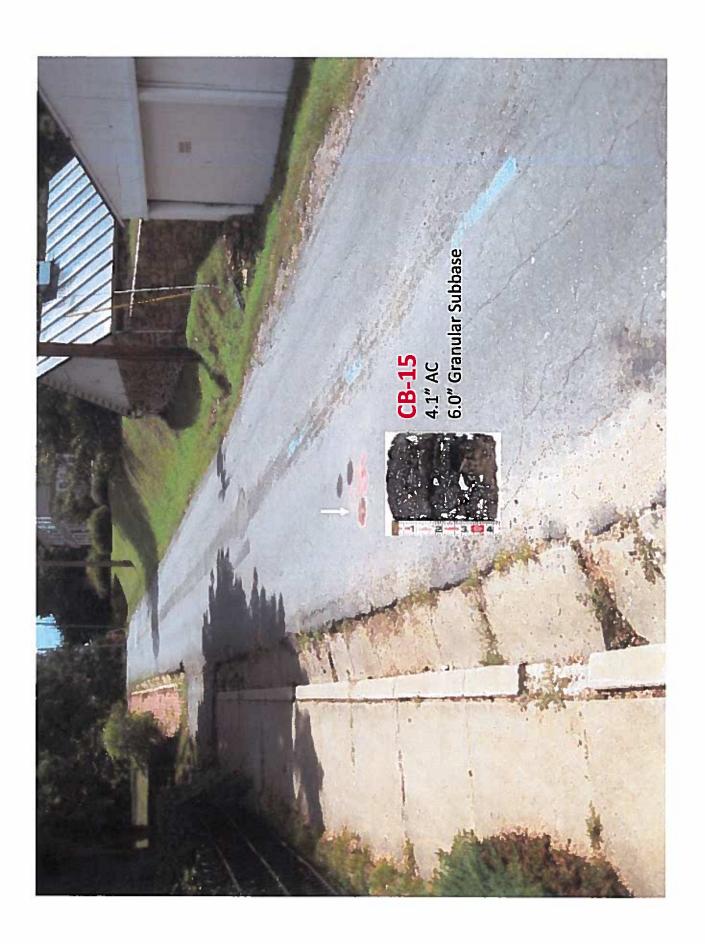


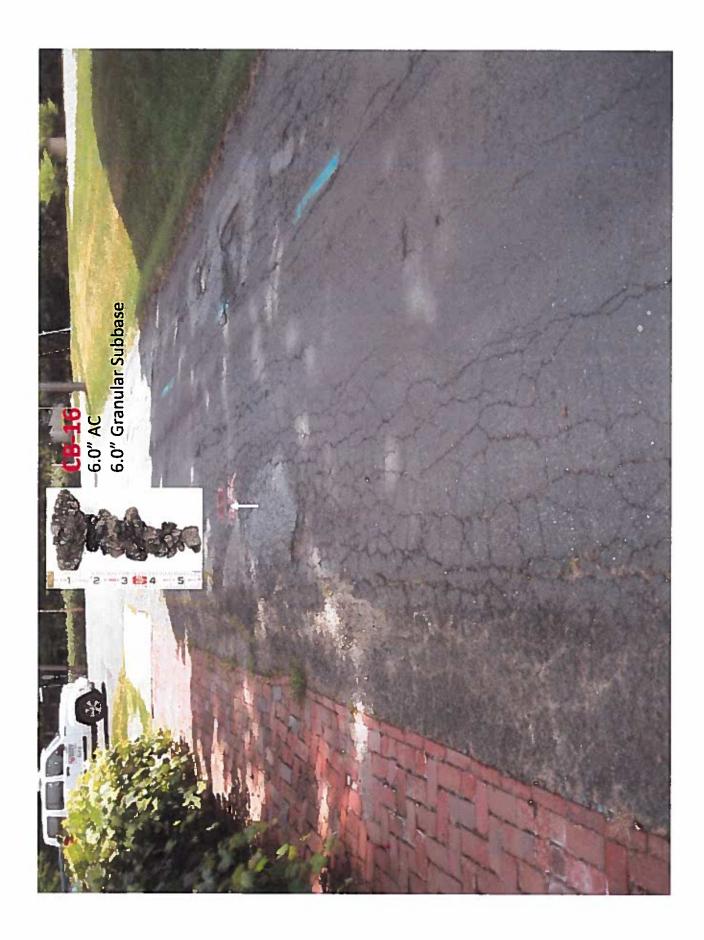


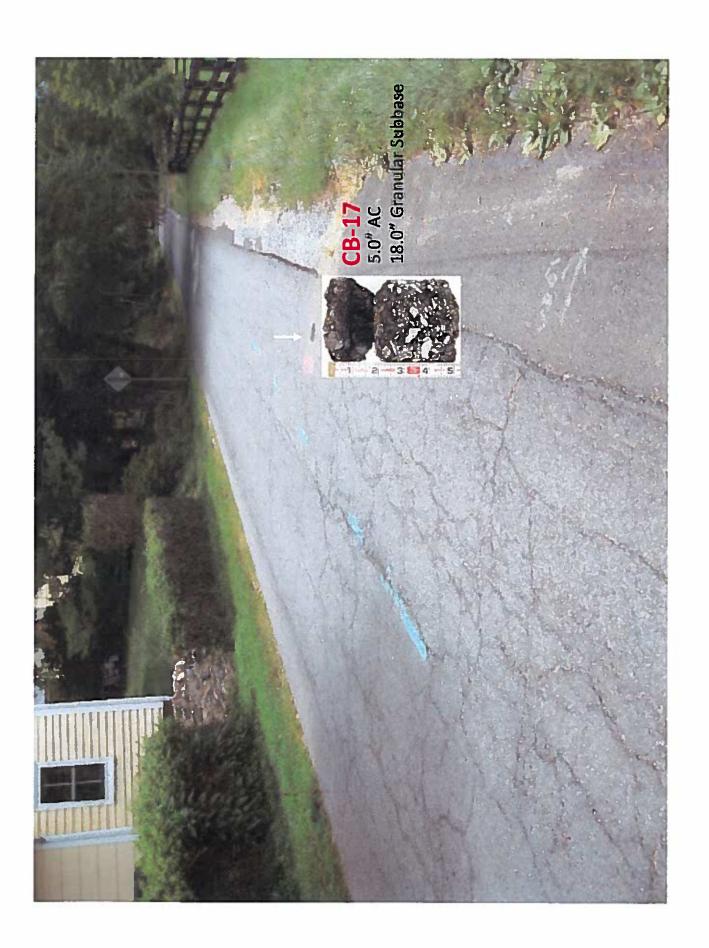


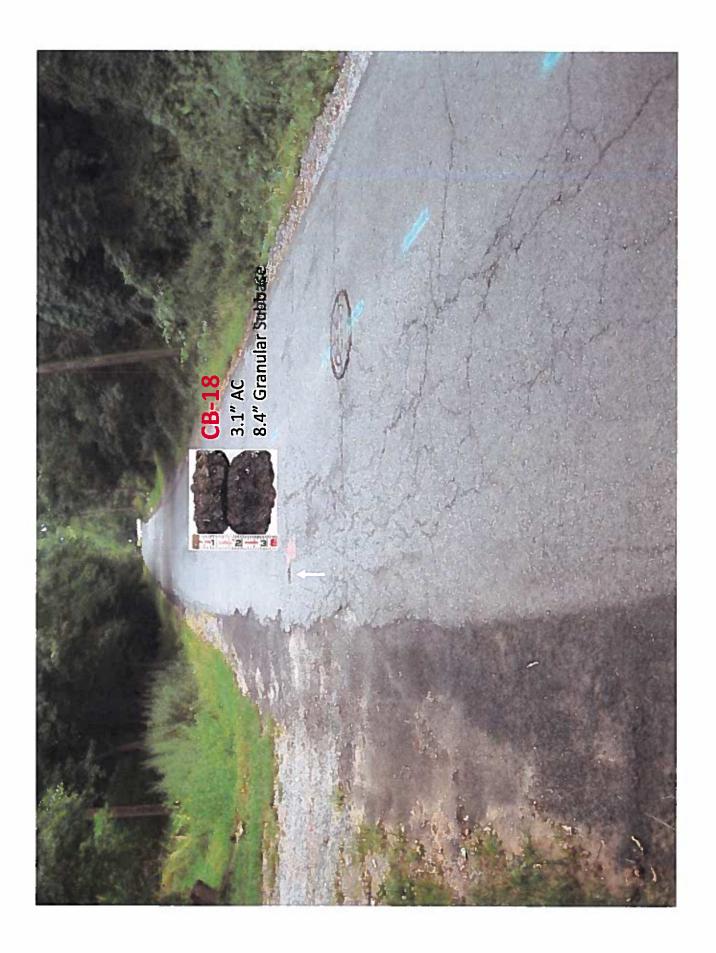


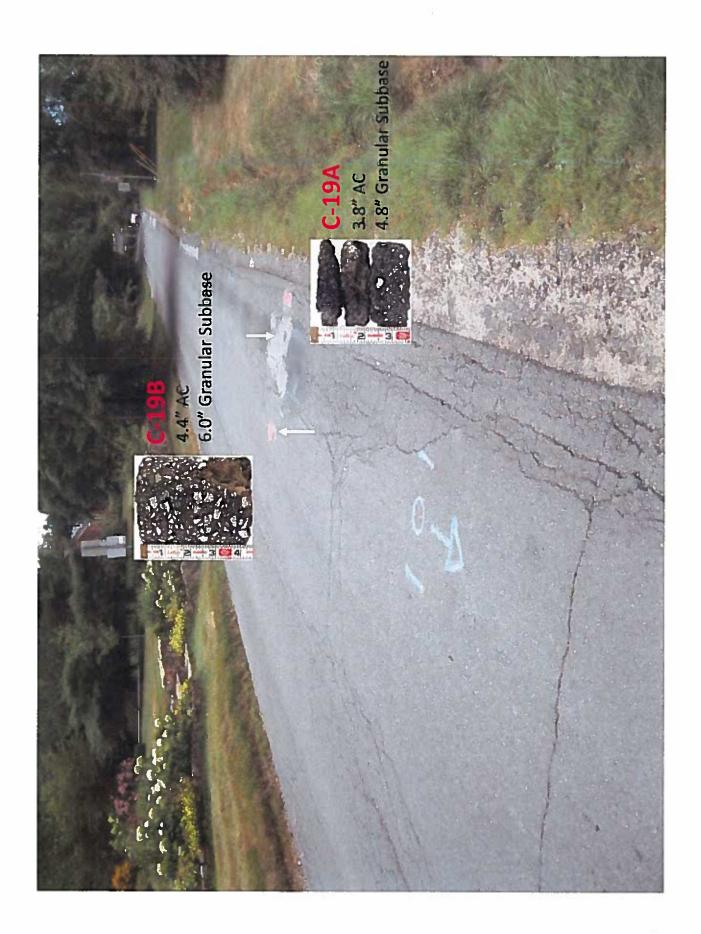




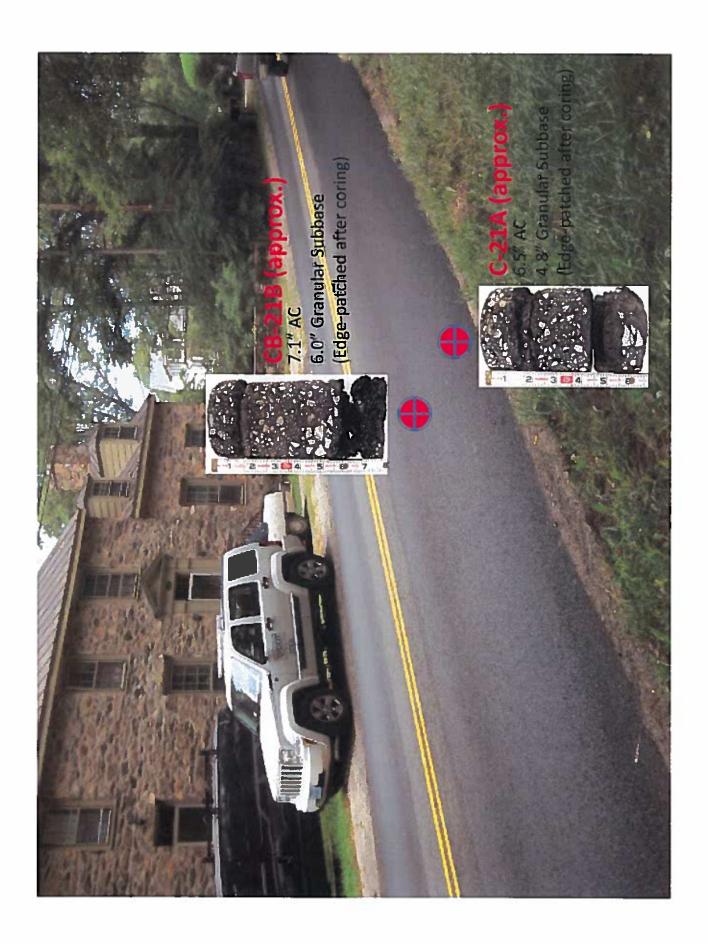












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DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY B	DI	JOINTS	STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING	LIQUID LIMIT	PLASTICITY INDEX	MOISTINGE CONTENT (%)
		<u> </u>	SO		0"	18	L -			"	NO LONG TERM MEASUREMENTS TAKEN			Įğ
4			-	1		Ľ					FIELD DESCRIPTION OF STRATA	LL	PI	Ļ
2	998	18 16 9	35		0.42						0.0 / 999.0 5.0" Asphalt Concrete 0.4 / 998.6 Fill, Brown and black CLAYEY F-C SAND AND FINE GRAVEL FILL, dense, moist to wet (SC/GC) 1.4 / 997.6 Residual, Red brown LEAN CLAY with f-c sand, stiff, moist (CL)			16
4 +	996	3 4 5 6 4 4 7	90		4.42				62528: 0		decrease in plasticity below 3.8'			25
		12		H	6.42						Boring Terminated at 6.4'			
- and the international and the state of the	Saludunindina Transfer Comment of the Comment of th		人。 一								Pavement Profile: 3.5" coarse surface/intermediate graded AC (4 layers) — top layer (0.8" thick) is substantially stripped — remaining three layers are completely stripped and friable. The 2nd (0.8" thick) and 4th (1.0" thick) layers disintegrated before or during coring. 1.5" surface graded AC (1 layer) — completely stripped with abundant binder NOTE: A vertical crack extends from the bottom of the AC thickness through all layers except the top coarse surface/intermediate layer.			
EM	VBK6	· Rig Type	CA	AE A	15C S	ikid.	Ric	Soc	anle	no.	2 was obtained from 2.0' to 6.4'.	<u></u>		Ļ
r=141	MKN2;	. Rig Type	:: UN	ri⊏ 4	10U S	KIO	rtig.	San	ipie	ΠO.	2 was obtained from 2.0' to 6.4'.	GE	<u>10</u>	F
												CE	1	

STATION: STA	4							7	\ <u> </u>			PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY	CE		_
FIELD DATA				z Usp			ci Y		5.72	n tha		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE:			
SOIL (B) HAD LAND BY A STAND BY			FI	ELD	ם נו	ATA									
998 12 112 12 10 75 10 996 4 8 75 8 76 8 76 8 77 8 996 4 8 76 8 76 8 77 8 996 7 7 8 997 8 998 12 10 998 12 10 998 12 10 998 12 10 998 12 10 998 12 10 998 12 10 998 12 10 998 12 10 998 12 10 10 10 10 10 10 10 10 10 10 10 10 10	DEPTH (ft)	ELEVATION (ft)	SOIL						DI	P°	STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)
998 12 10 17 10 18 19 19 10 17 10 17 10 17 18 18 18 18 18 18 18 18 18 18 18 18 18					Н							0.0 / 999.0	LL	PI	-
996 4 8 75 4.67 increase in gravel (quartz fragments) below 4.6' 7 6 6 8 Boring Terminated at 6.7'	2	998	12	75	V	0 67						0.7 / 998.3 Fill, Brown and black CLAYEY F-C SAND AND FINE GRAVEL FILL, medium dense, moist to wet (SC/GC) 1.7 / 997.3 Residual, Brown SILTY F-C SAND, trace fine gravel, medium			17.0
Pavement Profile: 1.3" coarse surface graded AC (1 layer) - substantially stripped 4.3" base graded AC (2 layers) - upper layer (2.7" thick) is mildly stripped - lower layer (2.7" thick) is mildly to moderately stripped with minor to moderate voids 2.4" surface graded AC (1 or 2 layers) - completely stripped. The lower 0.5" of the layer may represent	4	996	10.700	75	H	2.67			98			dense, moist (SM)			12.1
Pavement Profile: 1.3" coarse surface graded AC (1 layer) substantially stripped 4.3" base graded AC (2 layers) upper layer (1.6" thick) is mildly stripped lower layer (2.7" thick) is mildly to moderately stripped with minor to moderate voids 2.4" surface graded AC (1 or 2 layers) completely stripped. The lower 0.5" of the layer may represent	6	994		40		4.67						increase in gravel (quartz fragments) below 4.6'			21.8
Pavement Profile: 1.3" coarse surface graded AC (1 layer) substantially stripped 4.3" base graded AC (2 layers) upper layer (1.6" thick) is mildly stripped lower layer (2.7" thick) is mildly to moderately stripped with minor to moderate voids 2.4" surface graded AC (1 or 2 layers) completely stripped. The lower 0.5" of the layer may represent asphalt coated sand/gravel from the layer below.					H	6.67					3 2 3	Boring Terminated at 6.7'			
REMARKS: Rig Type: CMF 45C Skirl Rig. Sample no. 3 was obtained from 1.7 to 4.7' Sample no. 4 was obtained	REA	MARKS	The state of the s	CM	FA	50.5	Sin	Rin	San	nnle	no	1.3" coarse surface graded AC (1 layer) substantially stripped 4.3" base graded AC (2 layers) upper layer (1.6" thick) is mildly stripped lower layer (2.7" thick) is mildly to moderately stripped with minor to moderate voids 2.4" surface graded AC (1 or 2 layers) completely stripped. The lower 0.5" of the layer may represent asphalt coated sand/gravel from the layer below.			
REMARKS: Rig Type: CME 45C Skid Rig. Sample no. 3 was obtained from 1.7' to 4.7'. Sample no. 4 was obtained PAGE 1 C	REN	MARKS 1 4.7' to	: Rig Type 6.7'.	: CM	E 4	5C S	Skid	Rig.	San	nple	no.	3 was obtained from 1.7' to 4.7'. Sample no. 4 was obtained			
CB-2			_										CE	3-2)

	4							_				PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY	C	-3	4
- 1		M.									Į.	STRUCTURE: PAVEMENT P.	AG	E 1	OF 1
		1	1587	الله الله	-8-		T	neg.	12			STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATU	M:	NAD	83
Ī			FI	ELI	ם כ	ATA	\			_		Date(s) Drilled: 6/20/16	TL	ABI	DATA
	DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY B	DI	OINTS	STRATA LEGEND	Drilling Method(s): 4" ID Pvmt Core/Hand Auger SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN		LIQUID LIMIT	MOISTURE CONTENT (%)
-	\dashv				\mathcal{H}	_	-	-		_		FIELD DESCRIPTION OF STRATA		L P	<u> </u>
SPT LOG WATERFORD PVMT INVESTIGATION GPJ 8 30,004 02 1011,8/15/16	he belondere bestellt betalle					0.42						5.0" Asphalt Concrete 0.4 / 998.6 Fill, Dark brown to black F-C SAND FILL with fine gravel and clay, dense, moist to wet (SP/SC) 0.8 / 998.2 Residual, Dark brown SILTY F-C SAND, trace fine gravel, medium dense, wet (SM) Hand Auger Terminated at 0.9' Pavement Profile: 1.9" coarse surface graded AC (1 layer) — mildly stripped with moderate voids 3.1" intermediate graded AC (1 layer) — mildly stripped with minor to moderate voids			11.5
PT_LOG:WATERFOR	REM	IARKS:	Rig Type	: CN	<u> </u> 1E 4	5C S	ikid	Rig.				PA		E 1 (OF 1
			Commonweal	45 -43	Commi						_				*

	4							_	_	-		PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY	C-	_	
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			123	e Ga	101	turi.	112		112			STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATUM	Л: N.	AD 8	33
			FI	ELI	ם כ	ATA	\					Date(s) Drilled: 6/21/16	LA	B D/	ATA
	DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION	DI	JOINTS	STRATA LEGEND	Drilling Method(s): 4" ID Pvmt Core/Split-Spoon SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA	F LIQUID LIMIT	2 PLASTICITY INDEX	MOISTURE CONTENT (%)
SPT_LOG WATERFORD PVMT INVESTIGATION GPJ 8:30:004:021011 8/15/16	2	998 997	21 10 5 3	60		2.5					Establish and Angel Ange	Pavement Profile: 2.6" coarse surface graded AC (1 layer) — moderately stripped 2.5" coarse surface graded AC (1 layer) — moderately stripped 2.5" coarse surface graded AC (1 layer) — moderately stripped 0.9" surface graded AC (1 layer) — completely stripped. A vertical crack extends through the layer.			17.5
G.WATE	EM	ARKS	: Rig Type:	: CM	IE 4	5C S	kid	Rig.	-1			I PA	GE	1 0	F 1
		white Seco	, Commonwealt	da -7°	E								C-3		_

4					_			1 1		PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY	CE		
	1	(新	2 (22)		teef		7.72	Taria.		STRUCTURE: PAVEMENT STATION: LATITUDE: 36.999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM	GE		
Н		FI	ELD	DA'	—— ГА		_	-		Date(s) Drilled: 6/21/16	τ		ATA
ОЕРТН (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS		SAMPLE LEGEND	CORF RECOVERY (%)	Τ	\vdash	JOINTS	STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA	FIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)
2	998	27 39 15 8 3 4 6 6	35 90	2.4.4	6					0.0 / 999.0 5.0" Asphalt Concrete 0.5 / 998.5 Fill, Brown and black CLAYEY F-C SAND AND FINE GRAVEL FILL, very dense, moist to wet (SC/GC) 0.8 / 998.2 Fill, Brown SILTY F-C SAND FILL, trace fine gravel, very dense to medium dense, moist (SM) 2.5 / 996.5 Residual, Red brown LEAN CLAY with f-m sand, stiff, moist (CL) firm below 4.5'			29.1
REM	TARKS	Rig Type	CME	6.4		Ria				Pavement Profile: 2.0" coarse surface graded AC (1 layer) mildly to moderately stripped 1.5" intermediate graded AC (1 layer) completely stripped and friable with abundant binder 1.5" surface graded AC (1 layer) completely stripped and friable with abundant binder			
REM	MARKS:	Rig Type	: CME	450	Skid	Rig.					GE		
											CB	-4	

4	1										PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY	CE	3-5	5
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			- Ja	See 1	-100			è	_		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATUM	1: N	AD 8	33
		FI	IELE	ם כ	ATA	_					Date(s) Drilled: 6/21/16	LA	B D/	ATA
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	(%)	ROCK QUALITY DESIGNATION O	STRATA	_	STRATALEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)
-				\mathcal{H}							FIELD DESCRIPTION OF STRATA	LL	Pl	
2	998	18 10 7 5	75		2 42						5.0" Asphalt Concrete 0.4 / 998.6 Fill, Brown CLAYEY F-C SAND FILL with fine gravel, medium dense, moist (SC) 1.2 / 997.8 Fill, Brown SILTY F-C SAND FILL, trace fine gravel, medium dense, moist (SM) 2.4 / 996.6	-		14.
4	996	2 4 5	80	V			3				Residual, Red brown LEAN CLAY, trace fine sand, firm, moist (CL)			28.0
	994	6 11 16	100	M	4.42						very stiff below 4.4'			18.
6		22	2	A.	6.42					PATERIAL SECTION OF THE PATERI	5.6 / 993.4 Residual, Brown SILTY F-C SAND, trace fine gravel/rock fragments, medium dense, moist (SM) Boring Terminated at 6.4'			
Printelli bril i	telentificial property of the second of the					À					Pavement Profile: 1.4" coarse surface graded AC (1 layer) completely stripped. A vertical crack extends through the layer. The layer partially disintegrated before or during coring. 1.9" intermediate graded AC (1 layer) completely stripped. The layer partially disintegrated before or during coring. 1.7" surface graded AC (1 layer) completely stripped and friable with abundant binder. At least one diagonal crack extends through the layer. The layer partially disintegrated before or during coring.			
REM	ARKS:	: Rig Type	: CM	IE 45	5C S	kid F	Rig.	O _C				GE CB		

		\ /							17		PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY STRUCTURE: PAVEMENT P		-6/	
	1		a Ju		154	48 T	2 16	1172	127		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATU			0F 1
		FI	ELE	ם כ	ATA	_					Date(s) Drilled: 6/21/16	$\overline{}$		DATA
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION O	DI	JOINTS	STRATA LEGEND	Drilling Method(s): 4" ID Pvmt Core/Split-Spoon SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIFL D. DESCRIPTION OF STRATA		5	MOIS
2	998	12 6 6 5	85		2.3					Anna Anna Anna Anna Anna Anna Anna Anna	Pavement Profile: 0.8" carse surface graded AC (1 layer)		L Pi	17.3
REI	MARKS:	Rig Type	: CM	E 4	5C S	kid	Rig.				PA		1 C)F 1
<u></u>		Commonwealt											31	•

FIELD DATA FIELD DATA FIELD DATA SOIL SOIL SOIL SOUR BROCK OND TO SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA 16 998 16 998 16 998 STATION: LATITUDE: 36.9999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT LAB DAT LAB DAT A VAIVE LEGEND STATION: SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND STATION: LATITUDE: 36.9999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND STATION: SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND STATION: LATITUDE: 36.9999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND STATION: LATITUDE: 36.9999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND STATION: LATITUDE: 36.9999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND STATION: LATITUDE: 36.9999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND STATION: LATITUDE: 36.99999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND STATION: LATITUDE: 36.99999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 LAB DAT A VAIVE LEGEND SOIL LEGEND SO		A.	\/		7			7		PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY STRUCTURE: PAVEMENT	C- AGE	6E	
Date(s) Drilled: 6/21/16 Drilled: 6/21/16 Drilling Method(s): 4" ID Pvmt Core/Split-Spoon		1		e Sizo	giota o	4-11			-01.	STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE:			
SPT Method: Automatic Hammer Other Test(s): Other Test(s): Dip- Other Test(s): Other Test(s): Dip- Other T			FI	ELD	DA'	TA				Date(s) Drilled: 6/21/16	_	_	_
998 16 9 80 2 997 8 80 2 997 8 80 2 Pavement Profile: 2.1* coarse surface graded AC (1 layer) — mildy stripped with minor voids 2.2* surface/coarse surface graded AC (1 layer) — completely stripped. The upper 0.5* of the layer partially disintegrated before or during coring. 3.2* intermediate/base graded AC (1 layer) — completely stripped. The upper 0.5* of the layer partially disintegrated before or during coring. 3.** intermediate/base graded AC (1 layer)	DEPTH (ft)	ELEVATION (ft)		\Box	SAMPLE LEGEND	SAMPLE INTERVAL		T	DIP "	SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN		_	
998 16 9 8 8 9				\vdash					+	0.0 / 999.0		PI	╁
Pavement Profile: 2.1" coarse surface graded AC (1 layer) mildly stripped with minor voids 2.2" surface/coarse surface graded AC (1 layer) completely stripped. The upper 0.5" of the layer partially disintegrated before or during coring. 3.2" intermediate/base graded AC (1 layer)	2	930	9	80						0.62 / 998.38 Fill, Brown SILTY/CLAYEY F-C SAND FILL with fine gravel, medium dense, wet (SM/SC) 1.3 / 997.7 Fill, Brown SILTY F-C SAND FILL, trace fine gravel, medium dense, moist (SM) 1.9 / 997.1 Black ASPHALT PAVEMENT DEBRIS — probable disintegrated pavement			16
			16	Q.V.		13		1	1				
	:EM	հունին հունովանների հունովանի հունո	Rig Type	CME	450	C Ski	id R	ig.		2.1" coarse surface graded AC (1 layer) mildly stripped with minor voids 2.2" surface/coarse surface graded AC (1 layer) completely stripped. The upper 0.5" of the layer partially disintegrated before or during coring. 3.2" intermediate/base graded AC (1 layer) moderately stripped with moderate voids	GE	10	F

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		V									STATION: OFFSET:	AGE	<u>1 C</u>)F 1
	0.011	Thirt							100		LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATU	IM: N	AD 8	33
		FI	ELC	ם כ	ATA	\					Date(s) Drilled: 6/22/16		_	ATA
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	(%	ROCK QUALITY 20 DESIGNATION O	DII	STNIOL	STRATA LEGEND	Drilling Method(s): 4" ID Pvmt Core/Split-Spoon SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)
-	E	ST/ SNETF	JIL RE	SAM	SAME	RE R	ROCH	STE	ᅙ	STR/	NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN		₹	STUR
		1 2	S	\		8					FIELD DESCRIPTION OF STRATA	LL	PI	₹
										81	0.0 / 999.0 6.1" Asphalt Concrete			
2	998	8 9 6	55	$\sqrt{}$	0.55						0.5 / 998.5 +13.0" Crushed Aggregate			6.1
	331	6	ò		2.55						Boring Terminated at 2.6'; however, no sample recovery below 1.7'			
REA	ARKS	Rig Type	CM		5C S	kid	Rig.				Pavement Profile: 1.0" coarse surface graded AC (1 layer) mildly stripped with minor voids. A vertical crack extends through the layer. 5.1" coarse surface/intermediate graded AC (1 or 2 layers) mildly stripped lower 1.8" contains moderate voids	AGE	10	F 1
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	ight 2016.	Commonweal	th of V	/irgin	ia		_					3-1		

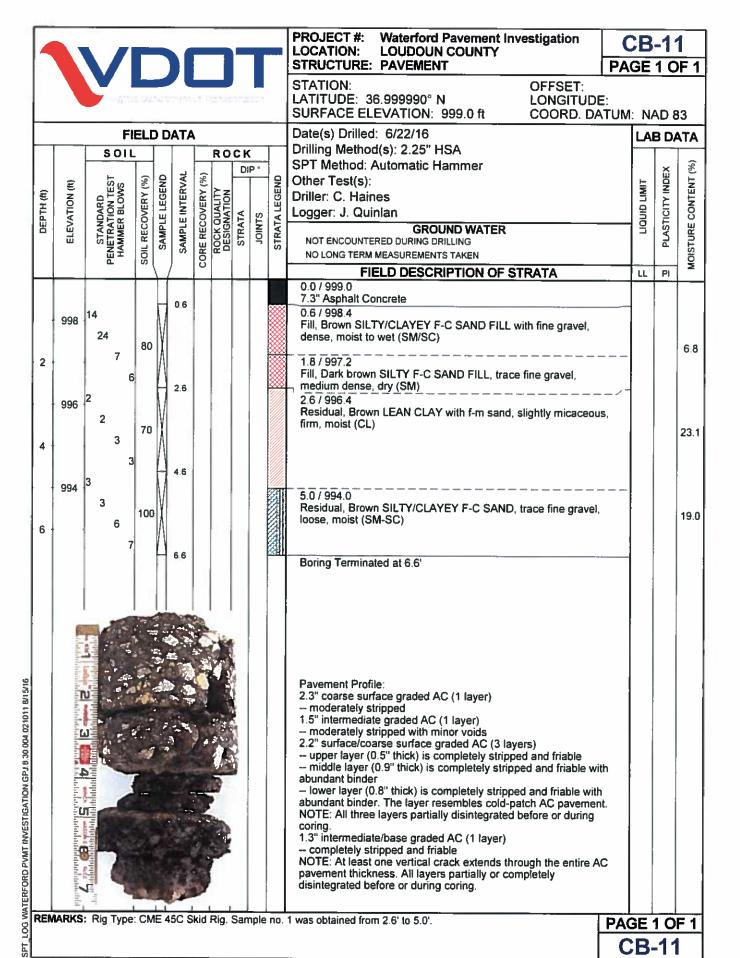
4		. /							1				7B	
			e Vien			110	12 12	007,00	1112		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE:	<u>GE</u>	<u>1 C</u>)F 1
\vdash											SURFACE ELEVATION: 999.0 ft COORD. DATUM	T .		
		SOIL	_) D.	ATA	\	RO	СК			Date(s) Drilled: 6/22/16 Drilling Method(s): 4" ID Pvmt Core/Split-Spoon	LA	B D/	ATA
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS		SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)		DI	STNIOL	STRATA LEGEND	SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)
		<u>a</u> _	8		,	8					FIELD DESCRIPTION OF STRATA	LL	PI	S S
2	998	16 19 12 11	70		0.62						0.0 / 999.0 7.1" Asphalt Concrete 0.6 / 998.4 Fill, Dark brown and black SILTY F-C SAND FILL with fine gravel and asphalt pavement debris, dense, wet (SM) 1.0 / 998.0 Fill, Brown SILTY F-C SAND FILL with fine gravel, dense, moist to wet (SM) Boring Terminated at 2.6'			8.3
SPI LOS WALERFORD PVM I INVESTIGATION GPJ 8 30 004 021011 BTISTI6	- shadarkarisalsin/tuknyadasinaharinaharinaharinaharinaharinaharinaharinaharinaharinaharinaharinaharinaharinah	Rig Type:	CM	E 4	5C S	Skid	Rig.				Pavement Profile: 2.6" coarse surface/intermediate graded AC (1 layer) mildly to moderately stripped 2.9" intermediate graded AC (1 layer) moderately stripped 0.8" surface graded AC (1 layer) substantially to completely stripped 0.8" coarse surface graded AC (1 layer) substantially to completely stripped			
	inht 2040	Commercia	n of to	Sepal-								C-7	7B	
Соруг	ig⊓i 2016	Commonwealt	ii OT Vi	irgini	el									

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			V		ز								PAG	3E	<u>1 0</u>)F 1
				o Sia		- ard	Ϋ́	w.yb	h-2	167		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DAT	T INA-	NI/	\ D	2
ŀ	-		FI	FI I) D	ATA	_	-				Date(s) Drilled: 6/24/16				ATA
ŀ		_	SOIL	_			<u>`</u>	RO	СК			Drilling Method(s): 4" ID Pvmt Core/Hand Auger	ŀ	LAC	אָט כּ	TAIA
ĺ				П		یا ا	3		DI			SPT Method: Automatic Hammer			Ä	(%)
	€	N (f)	TES	%) ∖.	GEN	ER	(3)	≿≾			BENE	Other Test(s): Driller: C. Haines		THE I	S	TEN I
1	DEPTH (ft)	ATIO	DAR	VER	ELE	Į.	OVE:	JATI	¥	ý	A LE(Logger: J. Quinlan		LIQUID LIMIT	Ë	CON
1	풉	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION	STRATA	JOINTS	STRATA LEGEND	GROUND WATER NOT ENCOUNTERED DURING DRILLING	一	음	PLASTICITY INDEX	MOISTURE CONTENT (%)
1			FRE	ioi ioi	ŝ	SA	S S	윤범	S	7	S	NO LONG TERM MEASUREMENTS TAKEN			<u>~</u>	JIST
L				S I	\setminus	<u>/</u>	ŏ					FIELD DESCRIPTION OF STRATA		LL	PI	ž
					H	0.34						0.0 / 999.0 \4.1" Asphalt Concrete	7-			
		i				0.94						0.3 / 998.7				20.4
					Ш							7.2" Crushed Aggregate	A			
												\0.9 / 998.1	71			
					Ш							Residual, Grey brown F-C SANDY SILT, trace fine gravel, stiff, moist to wet (ML)	II			!
					$\ $							Boring Terminated at 0.9'	٦	İ		
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115/1		種		÷.	2		,		3			B	ŀ			
0118	=1						74	1	N	N	-	Pavement Profile: 4.1" coarse surface graded AC (2 layers)				
4.02							챙			a de		upper layer (2.5" thick) contains minor to moderate voids		- }		
3000							1	,i	L			 lower layer (1.6" thick) is substantially to completely stripped. At least one vertical crack extends through the layer. The layer 				
3P.18	<u>- C</u>	ป ≣์			51	感	뼮	BT.			6	partially disintegrated before or during coring.				
NO			O NAME			1		7								
LIGAT								43								
WEST	-4	7≣,	4							1						
/MT II	=	<u> </u>								١.						
0 P																
P.F.O.																
SPT_LOG WATERFORD PVMT INVESTIGATION.GPJ.8 30 004.021011 8/15/16	EM	ARKS:	Rig Type	: CM	II 4	5C S	kid	Ria								
8			3 75-	10/2				"श्री"				<u> </u>	PAG			
	ODVO	nht 2016	Commonweal	th of V	(im»n	12								,-C	<u> </u>	

4								_			LOOK 11014.	CB		
		V										<u>GE</u>	<u>1 C</u>)F 1
			a Us	p. 1	1901	417	W.W.	77.2	1172		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATUM	1: N.	AD 8	33
		FI	ELI	D D	ATA	4				_	Date(s) Drilled: 6/22/16	LA	B D/	ATA
		SOIL					RO				Drilling Method(s): 2.25" HSA			~
		<u> </u>	<u>چ</u> ا	۵	Æ	₹		DI	P.	۵	SPT Method: Automatic Hammer Other Test(s):	١.	ă	8
€	N E	TES	(\$)	N.	ERV	<u>چ</u>	È₹			GEN	Driller: C. Haines	₹	Ž	
DEPTH (ft)	ATIO	DAR	jij	EE	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	8	VAL	¥	ço	4 LE	Logger: J. Quinlan	LIQUID LIMIT	Ē	8 8
필	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION	STRATA	JOINTS	STRATA LEGEND	GROUND WATER	19	PLASTICITY INDEX	MOISTURE CONTENT (%)
	ш	ENE:	등	Ϋ́	SA	뿚	준법	S	٦	ST	NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN		<u> </u>	IST
		<u>a.</u>	Š	\	/	8					FIELD DESCRIPTION OF STRATA	1	PI	ξ
				П							0.0 / 999.0	⇈	<u> </u>	
		7		Н	0.5					***	5.8" Asphalt Concrete 0.5 / 998.5	1		
	998	4		M							Fill, Brown SILTY F-C SAND AND FINE GRAVEL FILL, medium	1		
		5	65								\(\text{dense, moist (SM)} \\ 0.9 / 998.1			20.1
2		, ,		W							Residual, Grey brown F-C SANDY SILT, trace fine gravel, stiff,	į .		
		_	1	Н	2.5						moist (ML) very stiff below 2.5'			
,	996	7		M							very sun below 2,5			
		10	15	W										19.1
4		9		M										13.1
		7	'	Ш	4.5									
	994	7		И	7.3						4.5 / 994.5 Residual, Brown F-M SANDY LEAN CLAY, very stiff, moist (CL)			
	994	8		.W							Residual, DIOWITTHIN SANDT CEAN CLAT, Very Still, Moist (CL)			
		10	100	'M										16.3
6		12		W										
				H	6.5					2000	Boring Terminated at 6.5'	1		
			ĺ	Н							-			
	4000	n di			-6		3							
	£								1					
	3		-45°	. 56			1977							
90	= 5	19		25					10					
1.8/15	-9,	5		Ea .		. 9	9-1		7					
2101	<u>=N</u>		4	Š			ii	'n			Pavement Profile:			
98			ر ایسا		e Ì				1		1.8" coarse surface graded AC (1 laver)			
08	Ξů						10 mg	A			mildly stripped with minor voids. A diagonal crack extends through the layer.			
를	68	~	y					m	n		2.5" intermediate graded AC (1 layer)			
<u> </u>							1	看	F		moderately to substantially stripped the lower 0.8" of the layer is completely stripped and it partially			
	4			-				1			disintegrated before or during coring. The lower 0.8" may represent a layer of coarse-graded surface treatment (tar and			
NAVE:	1					٠,.	100		4		chip)			
<u> </u>	EUT-	here	· '**	,	. ,		ه ري	V 1			1.5" surface graded AC (1 layer) completely stripped			
[i_{i}							
	0		2	6	145	F	1		-					
PEA	IVBR6	Rig Type	C	I I	50 9	le id	Dic.	Soc.	3010		6 was obtained from 0.9' to 4.5'.			
SPI LOS WATERFURD PVM INVESTIGATION GPJ 8 30.004 02:011 8/15/16	u W.	g .ypc			J- 0	INIQ	. vig.	Jail	יטופ	HU.	171	GE		$\overline{}$
											C	B-	-8E	3_
Соруп	ght 2016.	Commonweal	Ith of \	√irgini	a									

STATION: LATITUDE: 36.999990° N SURFACE ELEVATION: 999.0 ft FIELD DATA Date(s) Drilled: 6/22/16 Drilling Method(s): 4" ID Pvmt Core/Split-Spoon SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA 11 STATION: LONGITUDE: COORD. DATUM: NAD 83 LAB DATA COUNTER: STATION: LONGITUDE: COORD. DATUM: NAD 83 LAB DATA COORD. DATUM: NAD 83 LAB DATA Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA 11 11 11 11 11 11 11 11 11	4		\/	-					1			PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY STRUCTURE: PAVEMENT	PAC	C.)F 1
FIELD DATA SOIL SOIL BOTHING Method (3): 4" ID Pwmt Core/Split-Spoon SPT Method. Automatic Harmmer Opport Test(9): BOTHING Method (2): 4" ID Pwmt Core/Split-Spoon SPT Method. Automatic Harmmer Opport Test(9): BOTHING METHOD WATER MOTOR Test Method. Automatic Harmmer Opport Test(9): BOTHING CALIFORNIA OPPORTUNING METHOD WATER MOTOR TEST METHOD WATER MOTOR TES		1	i degli de	757		CHI	-11	31750	10.758	372		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE:				
BEMARKS: Rg Type: CME 45C Shid Rig. STAMBLIAN LINE AND AND CONTRIBUTION OF STRATE AND CONTRIBUTION OF			FIE	ELD	D.	ATA	<u> </u>					Date(s) Drilled: 6/22/16				_
Pavement Profile: 3.2 coarse surface graded AC (2 layers) 4.75 Pavement Profile: 3.2 coarse surface graded AC (2 layers) Fill, Brown SILTY F-C SAND FILL, trace fine gravel, medium (dense, moist (SC)) 17.997.3 Residual, Brown SILTY F-C SAND, trace fine gravel, medium (dense, moist (SC)) 18.0 Pavement Profile: 3.2 coarse surface graded AC (2 layers) - upper layer (1.8" thick) is substantially stripped - lower layer (1.4" thick) is moderately stripped. The bottom of the first is suited such that it varies from 1.0" thick to 2.0" thick gravely are first intermediate graded AC (1 layer) - mildy stripped with numerous voids 1.2" surface graded AC (2 layers) - completely stripped 1.6" intermediate graded AC (1 layer) - completely stripped. The layer disintegrated before or during coring. PAGE 1 OF 1 C-9	DEPTH (ft)	ELEVATION (#)		SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)		DI	P.	STRATA LEGEND	SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA				MOISTURE CONTENT (%)
Pavement Profile: 3 2" coarse surface graded AC (2 layers) - upper layer (1.8" thick) is substantially stripped - lower layer (1.4" thick) is moderately stripped. The bottom of the layer is angled such that it varies from 1.0" thick to 2.0" thick across the core. 1.5" intermediate graded AC (1 layer) - mildy stripped with rumerous voids 1.2" surface graded AC (2 layers) - completely stripped 1.5" intermediate/base graded AC (2 layers) - completely stripped 1.6" intermediate/base graded AC (1 layer) - completely stripped. The layer disintegrated before or during coring.	2	330	11	75	\bigvee						STANDARD STA	0.0 / 999.0 7.5" Asphalt Concrete 0.6 / 998.4 Fill, Grey brown SILTY F-C SAND FILL with fine gravel, medium dense, moist (SM) 1.1 / 997.9 Fill, Brown CLAYEY F-C SAND FILL, trace fine gravel, medium dense, moist (SC) 1.7 / 997.3 Residual, Brown SILTY F-C SAND, trace fine gravel, medium	- / _			18.0
	DG:WATERFORD PVMT INVESTIGATION:GPJ:8:30.004:D21011:8/15/16	Sanda bacaa laaba kabaa kabaa kabaa kabaa kabaa kabaa kabaa kabaa kabaa kabaa kabaa kabaa kabaa kabaa kabaa ka	Rig Type:	CM	E 4	5C S	Skid	Rig.		別は日本の一名のでは、 では、 では、 では、 では、 では、 では、 では、		3.2" coarse surface graded AC (2 layers) upper layer (1.8" thick) is substantially stripped lower layer (1.4" thick) is moderately stripped. The bottom of the layer is angled such that it varies from 1.0" thick to 2.0" thick across the core. 1.5" intermediate graded AC (1 layer) mildly stripped with numerous voids 1.2" surface graded AC (2 layers) completely stripped 1.6" intermediate/base graded AC (1 layer) completely stripped. The layer disintegrated before or during coring.		SE	1 0	F1
	1 100		3 . , , , ,					·· 3'				H'				<u>r 1</u>
		right 2016	Commonwealt	h of Vi	irgini	a		550	į.					<u>ს-</u>	<u> </u>	

	/F					OCATION:		vestigation		-10	
	ana les	7040	1772	Spura	ST (5)	STATION: ATITUDE: 3	: PAVEMENT 36.999990° N LEVATION: 999.0 ft	OFFSET: LONGITUDI COORD. DA			
	FIELD I	DATA	<u> </u>			Date(s) Drille		000113.31		AB D	
ELEVATION (ft) STANDARD STANDARD	HAMMER BLOWS SOIL RECOVERY (%) SAMPLE LEGEND	SAMPLE INTERVAL	\$	DESIGNATION O	STNIOL STATES	PT Method: Other Test(s): Oriller: C. Hair ogger: J. Qu NOT ENCOUNTE NO LONG TERM	nes		i di di		MOISTURE CONTENT (%)
998 13	8 75	2.5				0.0 / 999.0 5.9" Asphalt C 0.5 / 998.5 Fill, Dark brow and asphalt pa no AC paver 1.6 / 997.4 Residual, Brov	oncrete on to black SILTY F-C SAND F evement debris, medium dense ment debris below 0.9' on CLAYEY/SILTY F-C SAND, fragments, medium dense, mo	ILL with fine grave, moist (SM)			12
Chairmann an taraint an landachadha hada ad an landachadhadha landachadhadhadhadhadhadhadhadhadhadhadhadhadh						upper layer (file: rface graded AC (2 layers) (1.6" thick) is mildly to moderat 4.3" thick) is mildly to moderat	tely stripped ely stripped			
REMARKS: Rig	Type: CME	45C S	kid Ri	ig.	<u> </u>	· · · · · · · · · · · · · · · · · · ·	<u> </u>		PAGE	10 -10	



4	A			7						EOCATION:		-1:	2)F 1
	1	Try ru	26420		ns di	Epitora)	RUTE	der.		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATUM		·	
		FI	ELD	DA	ΓA					Date(s) Drilled: 6/24/16	LA	B D	ATA
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS		SAMPLE LEGEND	CORE RECOVERY (%)	T		STNIOL	STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA	F LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)
			1 3						8	0.0 / 999.0	-	 	
2	998	8 6 7	60		5					6.0" Asphalt Concrete 0.5 / 998.5 Fill, Brown and grey SILTY F-C SAND FILL mixed with f-c crushed aggregate, medium dense, moist to wet (SM) 0.8 / 998.2 Fill, Brown LEAN CLAY FILL with f-m sand, stiff, moist (CL)			21.
4	996	4 3 7	55	2.	5					2.5 / 996.5 Fill, Brown to grey brown SILTY F-C SAND FILL, trace fine rock fragments, slightly micaceous, medium dense, moist (SM)			17.
6	994	2 3 4	100	4	5					4.5 / 994.5 Fill, Dark grey green LEAN CLAY FILL with f-m sand, trace organics (rootlets), moderate organic odor, firm, moist (CL)			25.
		6		6.	5				***	Boring Terminated at 6.5'			
	The state of the s									Pavement Profile: NOTE: The AC pavement disintegrated before or during coring. Pieces retrieved range from surface to base graded. All are completely stripped and friable.			
EN.	ARKS:	Rig Type	: CME	E 450	Skid	Rig.				PAG	3E	10	F
												-12	

4	1						7				LOOK TON: LOODOON OCCUPT		13 1 C	
	1		1.70	10.01	可能計	15	B P USTY	5727			STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATUM			
		FI	ELI) D.	ATA						Date(s) Drilled: 6/30/16	LA	B D	AT/
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS T	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	(%	ROCK QUALITY DESIGNATION O	DIF	JOINTS	STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA	F LIQUID LIMIT	PLASTICITY INDEX	
2	998	9 14 9 6	65	V	2.5						0.0 / 999.0 1.9" Asphalt Concrete 0.2 / 998.8 Fill, Brown and grey CLAYEY F-C SAND AND F-C GRAVEL FILL, medium dense, moist to wet (SC/GC) 1.0 / 998.0 Residual, Grey brown SILTY F-C SAND, trace fine gravel/rock fragments, medium dense, moist (SM)			14
4	994	5 8 11 15 15 15 15 15	65		4.5						dense below 4.5'			9
	The state of the s										Pavement Profile: 0.9" coarse surface graded AC (1 layer) — mildly stripped with moderate voids 1.0" surface graded AC (1 layer) — substantially stripped NOTE: A vertical crack extends through the entire AC pavement thickness.			
REM	IARKS:	Rig Type	: CM	E 4	5C S	kid f	Rig. S	Sam	ple	ΠQ.	5 was obtained from 1.0' to 6.5'.	GE	10	F
												B-		

4	1	V					7						13E	3)F 1
ÿ.	_	Madi	2 (78)	6.3	27.6		10.350	r i	K/ E		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATUM	1: N	AD 8	33
_			ELD	D/	ATA						Date(s) Drilled: 6/30/16 Drilling Method(s): 4" ID Pvmt Core/Split-Spoon	LA	B D	ATA
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	(%	ROCK QUALITY DESIGNATION O	STRATA	$\overline{}$	STRATA LEGEND	SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA	TIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)
2	998	6 6 7	75	M	0.5	18					0.0 / 999.0 6.0" Asphalt Concrete 0.5 / 998.5 Fill, Brown and grey CLAYEY F-C SAND AND F-C GRAVEL FILL, medium dense, moist to wet (SC/GC) 1.0 / 998.0 Residual, Brown SILTY F-C SAND with f-c rock fragments, medium dense, moist (SM) Boring Terminated at 2.5'	-	PI	13.0
	n w		10								Pavement Profile: 1.6" coarse surface graded AC (1 layer) mildly to moderately stripped 1.6" coarse surface/intermediate graded AC (1 layer) mildly stripped with numerous voids 2.8" base graded AC (1 layer?) NOTE: The layer completely disintegrated before or during coring. Pieces retrieved are completely stripped and friable.			
REM	MARKS:	Rig Type	: CM	E 45	sc s	kid F	Rig.				 		1 0 3E	

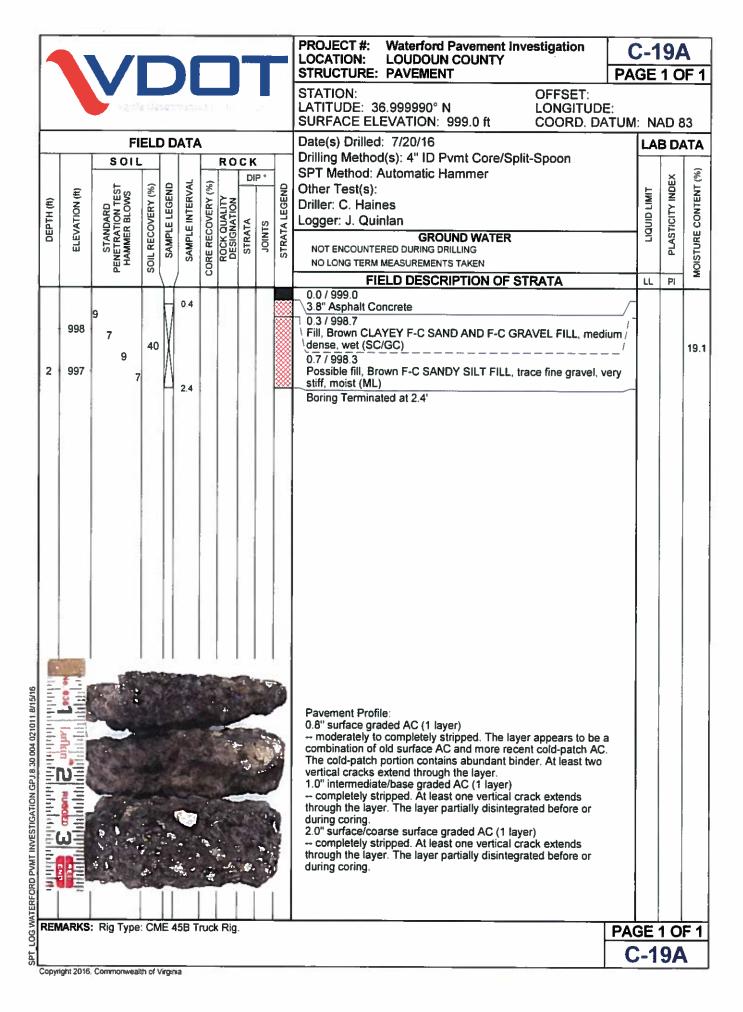
4												-14	_
		V		Ŋ				į		STRUCTURE: PAVEMENT PA STATION: OFFSET:	GE	1 C)F
			R LAGO		at Ta	2112	17,87	1677		LATITUDE: 36.999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM	I- NI	۸D (22
		FI	ELD	DATA		-	•			Date(s) Drilled: 6/30/16	_	B D	
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	\Box	SAMPLE LEGEND SAMPLE INTERVAL	(%	ROCK QUALITY BESIGNATION O	DIF	•	STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN	LIQUID LIMIT	PLASTICITY INDEX	THO THOU DO SOUTH
		-	\ \	_	٥			_		FIELD DESCRIPTION OF STRATA	LL	PI	Ž
2	996	4 21 35 29 20 21 35 27 32 50/6"	90	2.5						6.0" Asphalt Concrete 0.5 / 998.5 Fill, Grey and tan SILTY F-C SAND AND FINE GRAVEL FILL, loose to medium dense, moist (SM/GM) 0.7 / 998.3 Fill, Brown F-C SANDY SILT FILL with fine gravel, very stiff, moist (ML) 1.2 / 997.8 Residual, Light brown SILTY F-C SAND, trace fine rock fragments, very dense, dry (SM)			3.3
										Pavement Profile: 1.5" coarse surface graded AC (1 layer) moderately to substantially stripped. Two vertical cracks extend through the layer. The layer partially disintegrated before or during coring. 1.6" base graded AC (1 layer) completely stripped. The layer partially disintegrated before or during coring. 2.9" coarse surface graded AC (1 or 2 layers) completely stripped and friable with abundant binder. The layer partially disintegrated before or during coring.			
- 1	ARKS	: Rig Type	: CME	45C S	skid F	Rig.	-			l DA	GE	1.0	_
EM	AKNS:					-				I FA	JL		Г

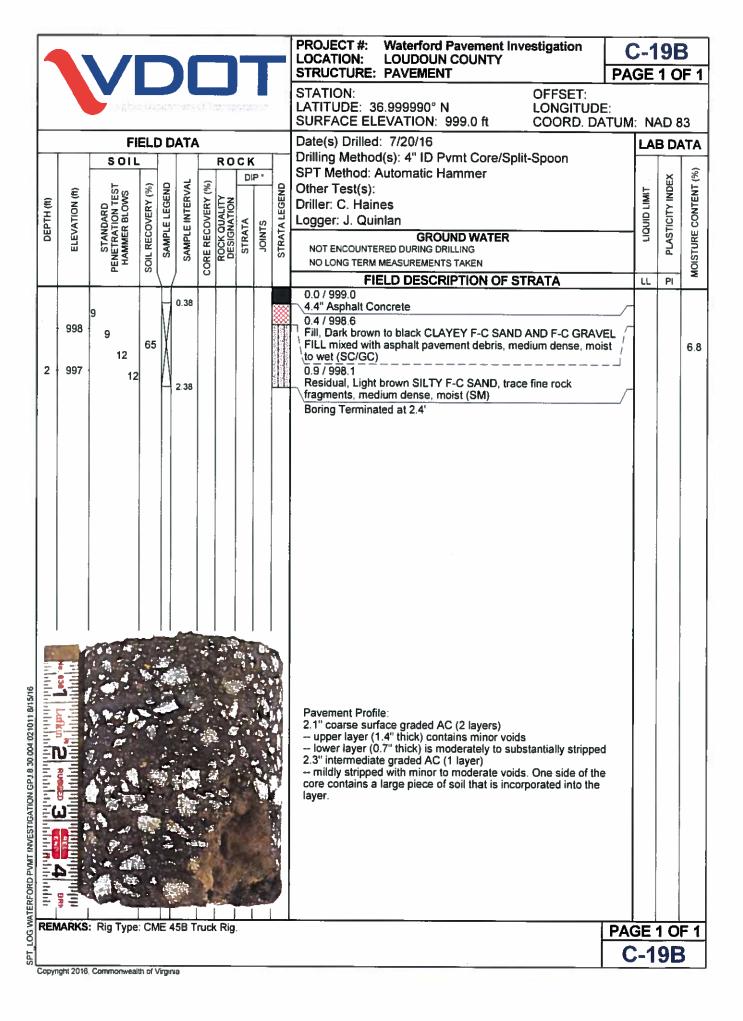
	4		\						\	1 [LOOK! LOODOO! COU!!	CB		_
		1		\$ 15k	爱 拉带	72.951	2FT		31 T.25	185		STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATUM			
			FI	ELC	ם כ	ATA	\					Date(s) Drilled: 6/24/16	T	_	ATA
	ОЕРТН (1)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	PESIGNATION D	DI		STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA	Е LIQUID LIMIT	T PLASTICITY INDEX	MOISTURE CONTENT (%)
	\dashv				П	0.37						0.0 / 999.0		PI	
	2	998	3 5 4 3	100	W W	2.37			:		<u></u>	4.1" Asphalt Concrete 0.4 / 998.6 Fill, Brown SILTY F-C SAND FILL with fine gravel, loose, moist (SM) 0.9 / 998.1 Residual, Brown SILTY F-C SAND, trace fine gravel, loose, moist (SM)			17.3
	4	996	2 4 5 5 5 6 5	100	\\ \\	4.37						decrease in sand content/size below 4.4'			26.4
					П	6.37					EAC 42	Boring Terminated at 6.4'			
ATERFORD PVMT INVESTIGATION	REM	ARKS	Rig Type	CM	——————————————————————————————————————	SC S		Rig	Sam	pple	no.	Pavement Profile: 0.5" surface graded AC (1 layer) — mildly to moderately stripped 3.6" intermediate graded AC (3 layers?) — all layers are completely stripped — lower 2.5" (lower 2 layers?) contain numerous voids and a few pieces of uncoated, coarse crushed aggregate.	G	1 0	F 1
T 10G.V	KEM	IARKS:	Rig Type	: CN	1E 4	5C S	ikid	Rig.	Sam	ple	по	177	GE ·		
	`amri	oht 2016	Commonweal	th of \	-	in				_			<u>, D.</u>	• 15	,

4	1										E00A11014:	CB		
		V			1						STATION: OFFSET:	\GE	10	<u> </u>
		11 12 12						1072			LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATUM	A⊹ KI	۸۵	02
		F	IEL		ATA	_		191.72		73	Date(s) Drilled: 6/24/16	_	B D	
		SOI				Ĺ	RO	СК		Г	Drilling Method(s): 2.25" HSA	<u> </u>		\top
	£	TS (0	(%	9	\AF	8		\vdash	P°	9	SPT Method: Automatic Hammer Other Test(s):	_	DEX	
€ H	NO!	NA TE	ERY (LEGE	TER	ÆRY	TON			EGE	Driller: C. Haines	LIQUID LIMIT	Ĭ.	
DEPTH (f)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION	STRATA	JOINTS	STRATA LEGEND	Logger: J. Quinlan GROUND WATER		PLASTICITY INDEX	
	ᇳ	S1 ENET HAMI	OIL R	SA	SAM	1 2 2	SE	S	೪	STE	NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN		3	
_		4	S	_/	_	ŏ					FIELD DESCRIPTION OF STRATA	LL	PI	
				Н	0.5						0.0 / 999.0 6.0" ASphalt Concrete			
-	998	4		M						***	0.5 / 998.5 Fill, Brown and black SILTY F-C SAND AND F-C GRAVEL FILL	-		
		3 8	90	X							\ with fine asphalt pavement debris, loose, moist to wet (SM/GM) / 1.0 / 998.0			11
2			7	N							Residual, Brown SILTY F-C SAND, trace fine gravet, medium dense, moist (SM)			
		3		H	2.5						2.5 / 996.5	-		
Ì	996	3	100	N							Residual, Brown SILTY F-C SAND, trace fine gravel, loose, moist (SM)			
4		4	100	'M										2
		:	5	И	4.5									
-	994	4		M							 decrease in sand content/size, medium dense below 4.5' 			
		7	90	IXI										14
6	1		8	N								l		
				H	6.5					84840	Boring Terminated at 6.5'	-		
	3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	High der letterstationer der der der der der der der der der d	5								Pavement Profile: 0.8" intermediate graded AC (1 layer) substantially to completely stripped NOTE: The remaining AC pavement thickness disintegrated before or during coring. Pieces retrieved are base graded, completely stripped, and friable.			
REM	ARKS:	Rig Type	e: CM	E 4	5C S	kid	Rig.	- 1			PA	GE	10)F
											<u>- </u>	CB		

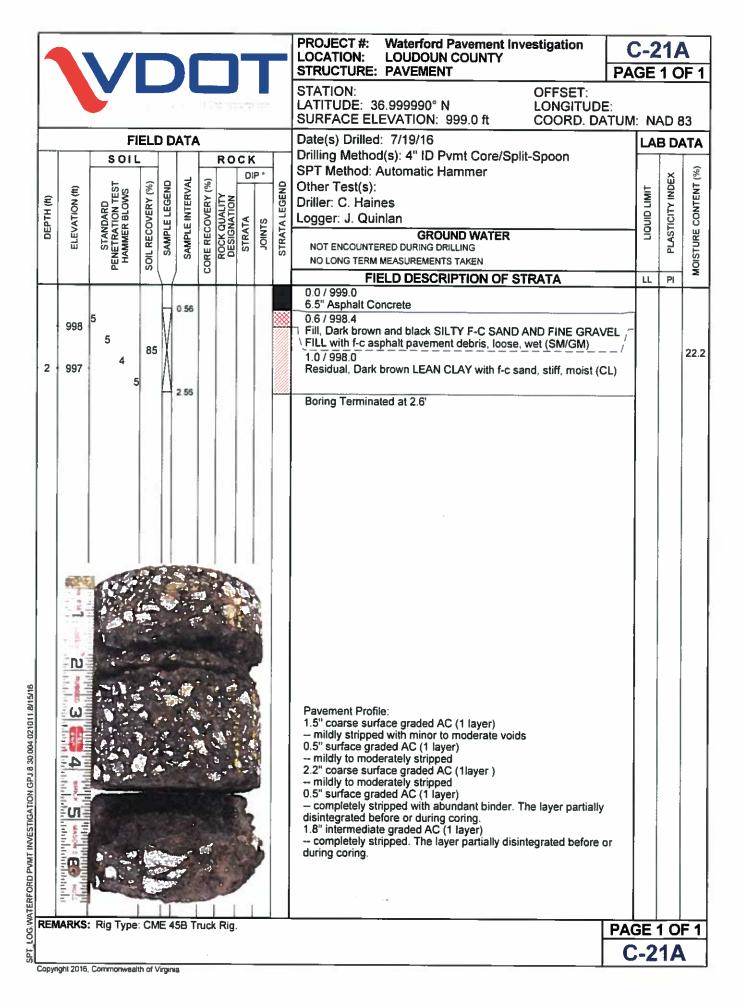
4	1										PROJECT #: Waterford Pavement Investigation LOCATION: LOUDOUN COUNTY STRUCTURE: PAVEMENT	CB		
			. /-		toje typ	2 7 7 2	Sparri Alah	H = 181	05.9		STATION: OFFSET:	AGE	10	<u> </u>
		E.O.H.		, Nr. 2	(Calle soll)	101.1	63.34P		20		LATITUDE: 36.999990° N SURFACE ELEVATION: 999.0 ft COORD. DATU	JM: N	AD :	83
			_	ם כ	ATA	_					Date(s) Drilled: 7/19/16	LA	B D	ΑТ
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	(%)	ROCK QUALITY DESIGNATION O	DI	JOINTS	STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER	LIQUID LIMIT	PLASTICITY INDEX	
	Ш	S. ENET	OIL R	SA	SAN	Z.	S E E	S	۲	ST	NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN		4	
	,	<u> </u>	ο (\ /	/	ၓ					FIELD DESCRIPTION OF STRATA	LL	PI	+
	998	13	50	V	0.5					***	0.0 / 999.0 5.0" Asphalt Concrete 0.4 / 998.6 Fill, Brown SILTY F-C SAND FILL with fine gravel, medium dense, moist to dry (SM)	<i></i>		
2	996	11 12 6 13			2.5					***	1.9 / 997.1 Residual, Brown SILTY F-C SAND with f-c rock fragments, medium dense, moist to dry (SM)			
6	994	13 16 12 36 46	85		4.5						light brown to tan, very dense below 4.5'			6
					6.5						Boring Terminated at 6.5'			
	The state of the s								一 一 一 一 一 一 一 一 一 一 一 一 一 一 一 一 一 一 一		Pavement Profile: 1.5" coarse surface graded AC (1 layer) — substantially to completely stripped. A vertical crack extends through the layer. The layer partially disintegrated before or during coring. 2.2" intermediate graded AC (1 layer) — substantially stripped with moderate voids. The upper 0.5" partially disintegrated before or during coring. 1.3" coarse surface graded AC (1 layer) — substantially stripped with minor voids. A vertical crack extends through the layer.			
REM		Rig Type	СМ	E 4	5B T	ruck	Rig.	-	1	97	P	AGE	10	F
												CB		

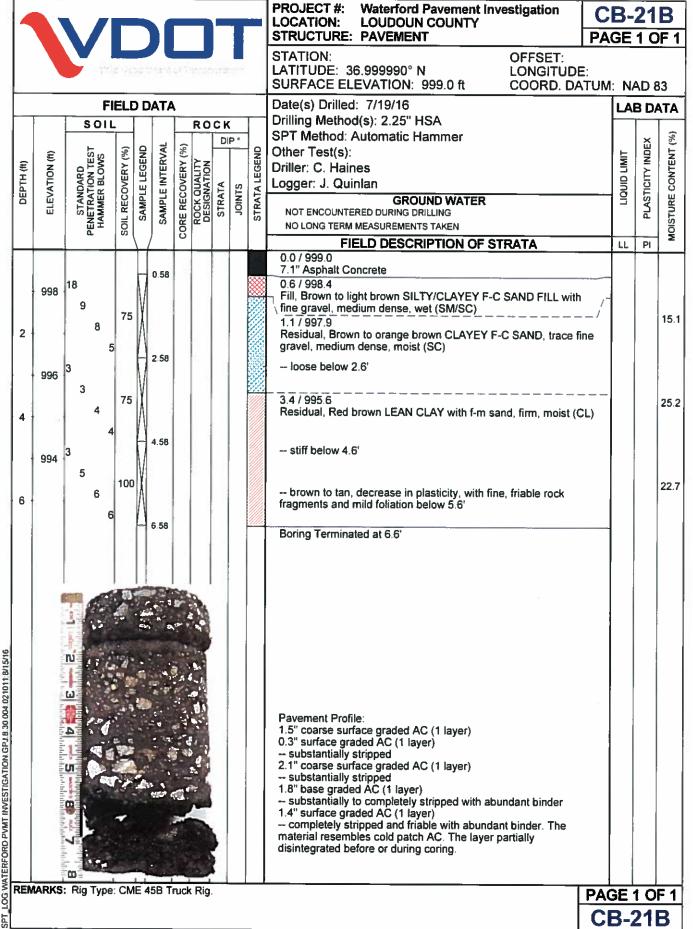
					Y							CB	_	
	1	Cara.	1,911	9011	ST (\$ 713)	(1) T	18-61	71720			STATION: OFFSET: LATITUDE: 36.999990° N LONGITUDE: SURFACE ELEVATION: 999.0 ft COORD. DATU			
		FI	ELC	ס כ	ATA						Date(s) Drilled: 6/30/16	LA	B D	ΑT
DEPTH (#)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLEINTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION O	DI	JOINTS	STRATA LEGEND	Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA	F LIQUID LIMIT	PLASTICITY INDEX	
2	998	14 18 14 16	55	V	2.42						0.0 / 999.0 3.1" Asphalt Concrete 0.3 / 998.7 Fill, Grey and brown CLAYEY F-C GRAVEL FILL with sand, dense, moist to wet (GC) decrease in fines below 0.7' 1.0 / 998.0 Residual, Brown SILTY F-C SAND, trace fine gravel, dense, moist to dry (SM) with f-c quartz fragments, medium dense below 2.4'			1
4	994	14 7 5 5 4	65 25	N	4.42						4.4 / 994.6 Residual, Brown CLAYEY F-C SAND, trace fine gravel, medium dense, moist (SC)	-		1
		7			6.42		200 00 00				Boring Terminated at 6.4'			
	The philipping of the state of										Pavement Profile: 1.3" coarse surface graded AC (1 layer) — moderately to substantialty stripped 1.8" surface/coarse surface graded AC (1 layer) — substantially to completely stripped NOTE: At least one vertical crack extends through the entire AC pavement thickness. Both layers partially disintegrated before or during coring.			
) DEN	ARKS	: Rig Type	: CM	E 4	5C S	kid	Rig.				PA	GE	10	F
CEIV												CB		





STATION: LATITUDE: 36.999990° N SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 FIELD DATA SOIL ROCK SURFACE ELEVATION: 999.0 ft COORD. DATUM: NAD 83 Date(s) Drilled: 7/19/16 Drilling Method(s): 2.25" HSA SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan WortenCounterRed During Drilling Not encounterRed During Drilling Not encounter Red Drilling Not encounter Red Drilling Not encounter Red Drilling Not e			LOCATION. LOODOON COONT	CB-2	
SOIL SOIL	FIELD DATA	el Antorone y acc	STATION: LATITUDE: 36.999990° N SURFACE ELEVATION: 999.0 ft OFFSET: LONGITUDE: COORD. DATUM	M: NAD	83
998 11 11 15 70 21 11 15 70 22 35 36 37 43* Asphalt Concrete 0.4 / 998.6 1 Fill. Brown grey SILTY F-C SAND AND FINE GRAVEL FILL. 1 medium dense, wet (SM/GM) 0.7 / 998.3 Residual, Brown and orange brown FINE SANDY SILT, slightly micaceous, very stiff, moist (ML) - increase in sand content/size, stiff below 2.4' - with fine, friable rock fragments and thin, black oxide stringers, with moderate schistosity, very stiff below 4.4' Boring Terminated at 6.4' Pavement Profile: 1.5' coarse surface graded AC (1 layer) - mildly stripped 2.0' surface graded AC (2 layers) - upper layer (0.6' thick) is completely stripped, friable, and with abundant binder. The layer disinlegrated before or during coring, - lower layer (1.4" thick) is completely stripped. 0.4 998.6 Pavement Profile: 1.5' coarse surface graded AC (2 layers) - upper layer (0.6' thick) is completely stripped.		DIP*	SPT Method: Automatic Hammer Other Test(s): Driller: C. Haines Logger: J. Quinlan GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN FIELD DESCRIPTION OF STRATA		MOISTLIRE CONTENT (%)
Pavement Profile: 1.5	2 998 11 70 70 235		4.3" Asphalt Concrete 0.4 / 998.6 Fill, Brown grey SILTY F-C SAND AND FINE GRAVEL FILL, medium dense, wet (SM/GM) 0.7 / 998.3 Residual, Brown and orange brown FINE SANDY SILT, slightly micaceous, very stiff, moist (ML)		20
Pavement Profile: 1.5" coarse surface graded AC (1 layer) - mildly stripped 2.0" surface graded AC (2 layers) - upper layer (0.6" thick) is completely stripped, friable, and with abundant binder. The layer disintegrated before or during coring. - lower layer (1.4" thick) is completely stripped. 0.8" base graded AC (1 layer)	5 7 5 994 8 70 12 6		with moderate schistosity, very stiff below 4.4'		9.
REMARKS: Rig Type: CME 45B Truck Rig. PAGE 1 OF			Pavement Profile: 1.5" coarse surface graded AC (1 layer) mildly stripped 2.0" surface graded AC (2 layers) upper layer (0.6" thick) is completely stripped, friable, and with abundant binder. The layer disintegrated before or during coring. lower layer (1.4" thick) is completely stripped. 0.8" base graded AC (1 layer) completely stripped		





Summary of Soil Laboratory Test Data

Waterford Pavement Investigation Project No. UPC No. 100418

Sample	Boring	Depth	Laboratory Soil Description	nscs	% Retained on	l .	QQW	OMC	Corr.	Corr. OMC	Atter	Merberg Limits	<u> </u>	CBH	Swell
Ž.				Class.	NO. 4 SIBVE	No. 200 Sieve	(bct)	8	(bct)	8	1	P.	<u></u>	(8-M-8)	2
1	CB-11	2.6" to 5.0"	CB-11 2.6' to 5.0' Brown LEAN CLAY with f-m sand, slightly micaceous	ŭ	0:0	7.97	104.9	21.2	NCA	NCB	33	ଥ	5	6.0	0.7
2	CB-11	2.0' to 6.4'	2.0' to 6.4' Red brown LEAN CLAY with 1-c sand	ರ	2.0	82.6					98	24	15		
3	CB-2	1.7 to 4.7	1.7 to 4.7 Brown StLTY F-C SAND, trace fine gravel	SM	9.0	44.9	121.3	12.0	NCB	NCR	₽ E	핥	P	12.0	0.5
4	CB-2	4.7 to 6.7	Brown CLAYEY F-C SAND, trace line gravel	SC	15.5	46.B					93	ನ	Ξ	-	
5	CB-13A	1.0' to 6.5'	Grey brown SILTY F-C SAND, trace fine gravel	SM	7.5	44.2	121.1	11.8	NCR	NCH	Ā	Ð	₽.	11.4	0.5
9	CB-BA	0.9' to 4.5'	0.9' to 4.5' Grey brown F-C SANDY SILT, trace fine gravel	ML	6.0	54.1			L		31	24	7		
7	CB-15	1.0' to 6.4	1.0' to 6.4 Brown SILTY F-C SAND, trace fine grave)	SM	66	40.0	118.7	12.8	NCA	NCA	Ā	ď	٩	10.0	9.0

Notes: 1. Soil dassification and teating in accordance with USCS, ASTM D2487, D421, D422, D4318, US COE EM1110, VTM-1 and VTM-8

2. MDD = Maximum Dry Density (VTM-1), not corrected for material retained on #4 sieve
OMC = Optimum Moisture Content (VTM-1), not corrected for material retained on #4 sieve
Corr. MDD and Corr. OMC are Maximum Dry density and Optimum Moisture Content corrected for 10% or more material retained on #4 sieve in accordance with VTM-1
NCR = No Correction Required to the MDD or OMC
CBR = California Bearing Ratio (VTM-8)

Atterberg Limits: c

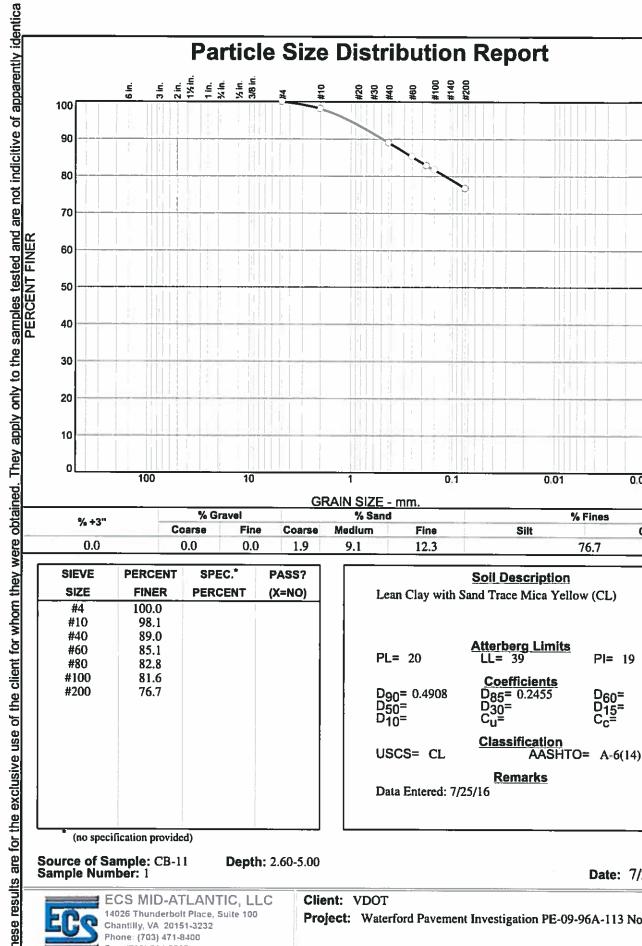
LL = Liquid Limit Pt = Plasticity Index

PL = Plastic Limit NP = Non-plastic

O:WattGINT LOGS (SPT Logs) & Geology Plan Sheets/Waterford Pvmt Investigation/Lab data/Waterford VDOT lab summary.xlsx

These results are for the exclusive use of the client for whom they were obtained. They apply only to the samples tested and are not indicitive of apparently identical LIQUID AND PLASTIC LIMITS TEST REPORT 60 Dashed line indicates the approximate upper limit boundary for natural soils CHOTOH 50 40 PLASTICITY INDEX 30 CIPOLO1 20 10 CL-ML ML or OL MH or OH 20 30 60 40 70 80 90 100 110 LIQUID LIMIT **MATERIAL DESCRIPTION** LL PL PΙ %<#40 %<#200 USCS Lean Clay with Sand Trace Mica Yellow (CL) 39 20 19 89.0 76.7 CL Project No. 24078-V Client: VDOT Remarks: ●Data Entered: 7/25/16 Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418 Source of Sample: CB-11 Depth: 2.60-5.00 Sample Number: 1 ECS MID-ATLANTIC, LLC 14026 Thunderbolt Place, Suite 100 Chantilly, VA 20151-3232 Phone: (703) 471-8400 Fax (703) 834-5527 **Figure**

Tested By: HTN1 Checked By: DVT



Source of Sample: CB-11 Sample Number: 1

Depth: 2.60-5.00

Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

0

10

20

30

40

50

60

70

80

90

100

0.001

Clay

PI= 19

D₆₀=

Date: 7/21/16

PERCENT COARSER

Project No: 24078-V **Figure**

ECS MID-ATLANTIC, LLC 14026 Thunderbolt Place, Suite 100

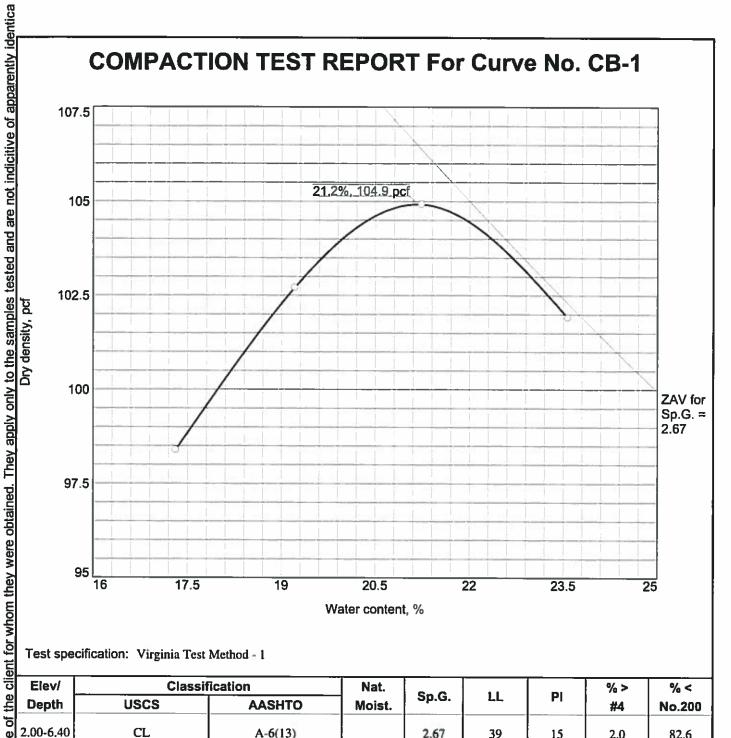
Chantilly, VA 20151-3232 Phone: (703) 471-8400 Fax: (703) 834-5527

Client: VDOT

Tested By: KV

Checked By: DVT

COMPACTION TEST REPORT For Curve No. CB-1



Test specification: Virginia Test Method - 1

Elev/	Classif	ication	Nat.	00		D.	% >	% <
Depth	USCS	AASHTO	Moist.	Sp.G.	<u>L</u> L	PI	#4	No.200
2.00-6.40	CL	A-6(13)		2.67	39	15	2.0	82.6

USE		11-0(13)	2,07	39	1.5	2.0	62,0
sive	1	TEST RESULTS	-	М	ATERIAL	DESCRIP	TION
exclu	Maximum dry density = 104.	9 pcf		Lean Cl	lay with Sai	nd Strong Bi	rown (2.0%
je L	Optimum moisture = 21.2 %					•	

Project No. 24078-V Client: VDOT

Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

Data Entered: 7/25/16 Date: 7/22/16

Remarks:

Source of Sample: CB-1 Sample Number: 2

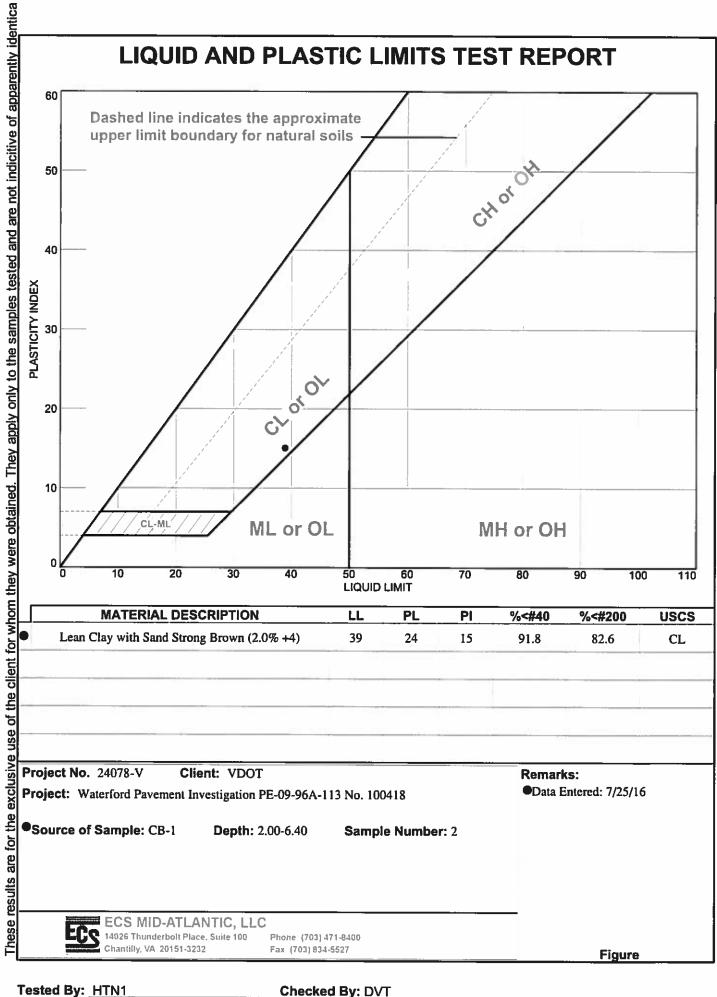
ECS MID-ATLANTIC, LLC 14026 Thunderbolt Place, Suite 100

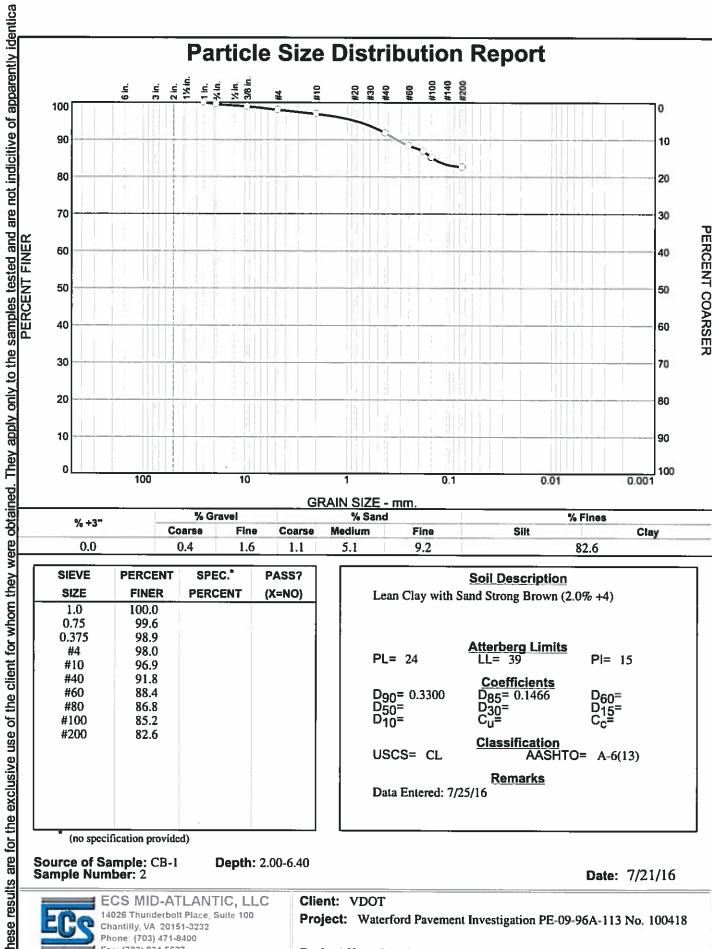
Chantilly VA 20151-3232

Phone: (703) 471-8400 Fax: (703) 834-5527

Figure

Tested By: KV





SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
1.0	100.0		
0.75	99.6		
0.375	98.9		
#4	98.0		
#10	96.9		
#40	91.8		
#60	88.4		
#80	86.8		
#100	85.2		
#200	82.6		
		557	

	Soil Description	
Lean Clay with S	Sand Strong Brown (2	.0% +4)
PL= 24	Atterberg Limits LL= 39	Pl= 15
	Coefficients	
D ₉₀ = 0.3300	D ₈₅ = 0.1466	Den=
D50=	D30=	D ₆₀ = D ₁₅ = C _c =
D ₁₀ =	C _u =	C _c =
	Classification	_
USCS= CL	AASHT	O= A-6(13)
	Remarks	
Data Entered: 7/2	25/16	
<u></u>		

* (no specification provided)

Source of Sample: CB-1 **Sample Number:** 2

Depth: 2.00-6.40

Date: 7/21/16



ECS MID-ATLANTIC, LLC

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Phone: (703) 471-8400 Fax! (703) 834-5527

Client: VDOT

Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

Project No: 24078-V

Figure

Tested By: KV

for the exclusive use of the client for whom they were obtained. They apply only to the samples tested and are not indicitive of apparently identica COMPACTION TEST REPORT For Curve No. CB-2 127 122 12,0%, 121.3 pcf 117 Dry density, pcf 112 ZAV for Sp.G. = 2.60 107 102 6.5 8.5 10.5 12.5 14.5 16.5 18.5 Water content, % Test specification: Virginia Test Method - 1 Elev/ Classification Nat. % > % < Sp.G. LL PΙ **USCS** Depth **AASHTO** Moist. #4 No.200 1.70-4.70 SM A-4(0)2.6 NP NP 9.0 44.9 **TEST RESULTS MATERIAL DESCRIPTION** Silty Sand Dark Brown (9.0% +4) Maximum dry density = 121.3 pcfOptimum moisture = 12.0 % Client: VDOT Project No. 24078-V Remarks: Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418 Data Entered: 7/25/16 Date: 7/22/16 Source of Sample: CB-2 Sample Number: 3 ECS MID-ATLANTIC, LLC

Tested By: KV Checked By: DVT

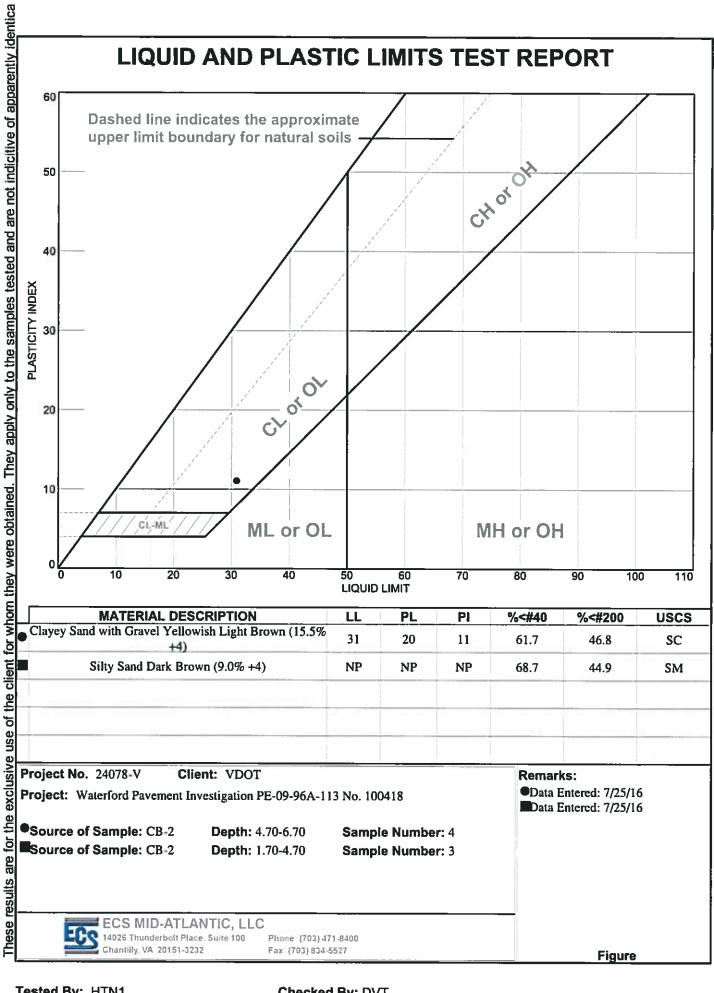
Phone: (703) 471-8400

Figure

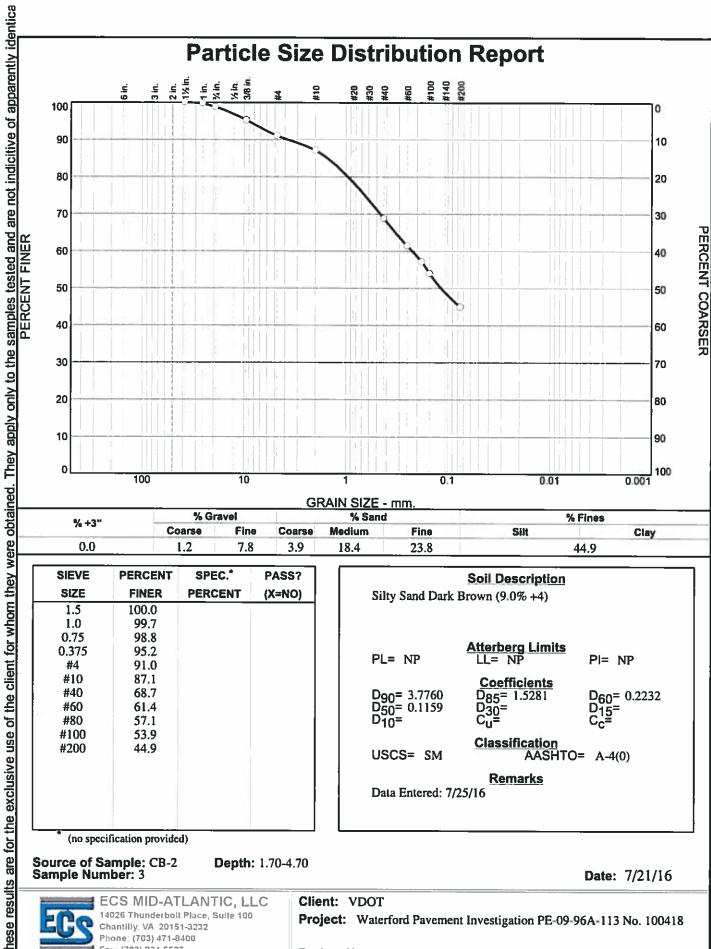
Fax: (703) 834-5527

🗨 14026 Thunderbolt Place, Suite 100

Chantilly, VA 20151-3232



Tested By: HTN1



SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
1.5	100.0		
1.0	99.7		
0.75	98.8		
0.375	95.2		
#4	91.0		
#10	87.1		
#40	68.7		
#60	61.4		
#80	57.1		
#100	53.9		
#200	44.9		
* ,			

Soil Description	
Brown (9.0% +4)	
Atterberg Limits LL= NP	PI= NP
Coefficients D85= 1.5281 D30= Cu=	D ₆₀ = 0.2232 D ₁₅ = C _c =
Classification AASHT	O= A-4(0)
Remarks 25/16	
	Coefficients D85= 1.5281 D30= Cu= Classification AASHT

(no specification provided)

Source of Sample: CB-2 **Sample Number:** 3

Depth: 1.70-4.70

Date: 7/21/16



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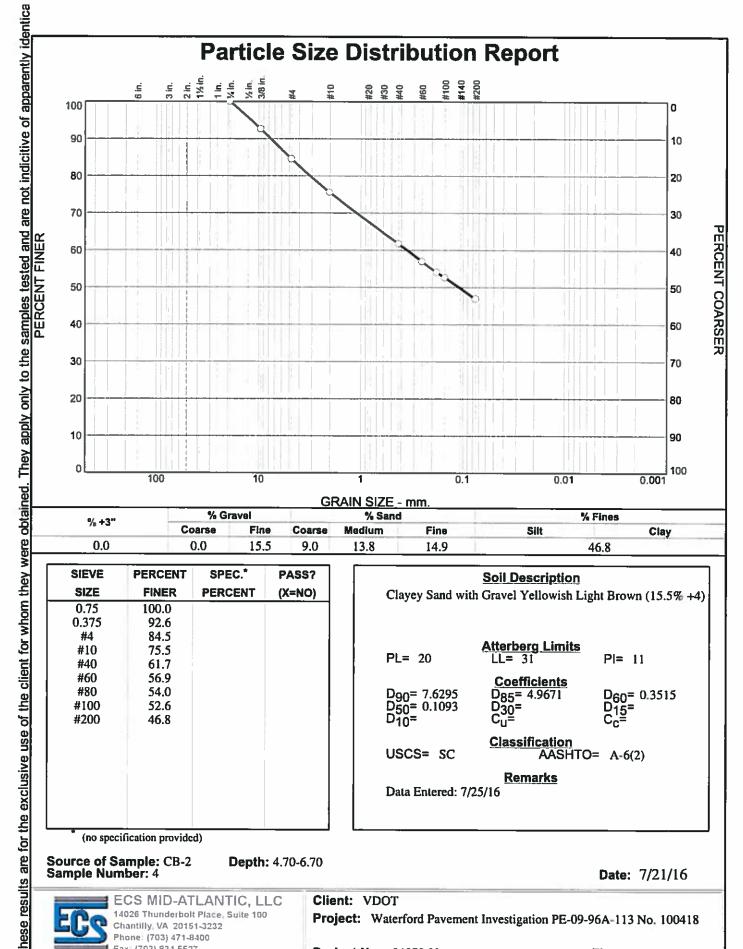
Client: VDOT

Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

Project No: 24078-V

Figure

Tested By: KV



SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
0.75	100.0		
0.375	92.6		
#4	84.5		
#10	75.5		
#40	61.7		
#60	56.9		
#80	54.0		
#100	52.6		
#200	46.8		

Clayey Sand wit	Soil Description h Gravel Yellowish I	Light Brown (15.5% +4)
PL= 20	Atterberg Limits	Pl= 11
D ₉₀ = 7.6295 D ₅₀ = 0.1093 D ₁₀ =	Coefficients D ₈₅ = 4.9671 D ₃₀ = C _u =	D ₆₀ = 0.3515 D ₁₅ = C _c =
USCS= SC	Classification AASHT	O= A-6(2)
Data Entered: 7/3	Remarks 25/16	
	_	

Source of Sample: CB-2 **Sample Number:** 4

(no specification provided)

Depth: 4.70-6.70

Date: 7/21/16



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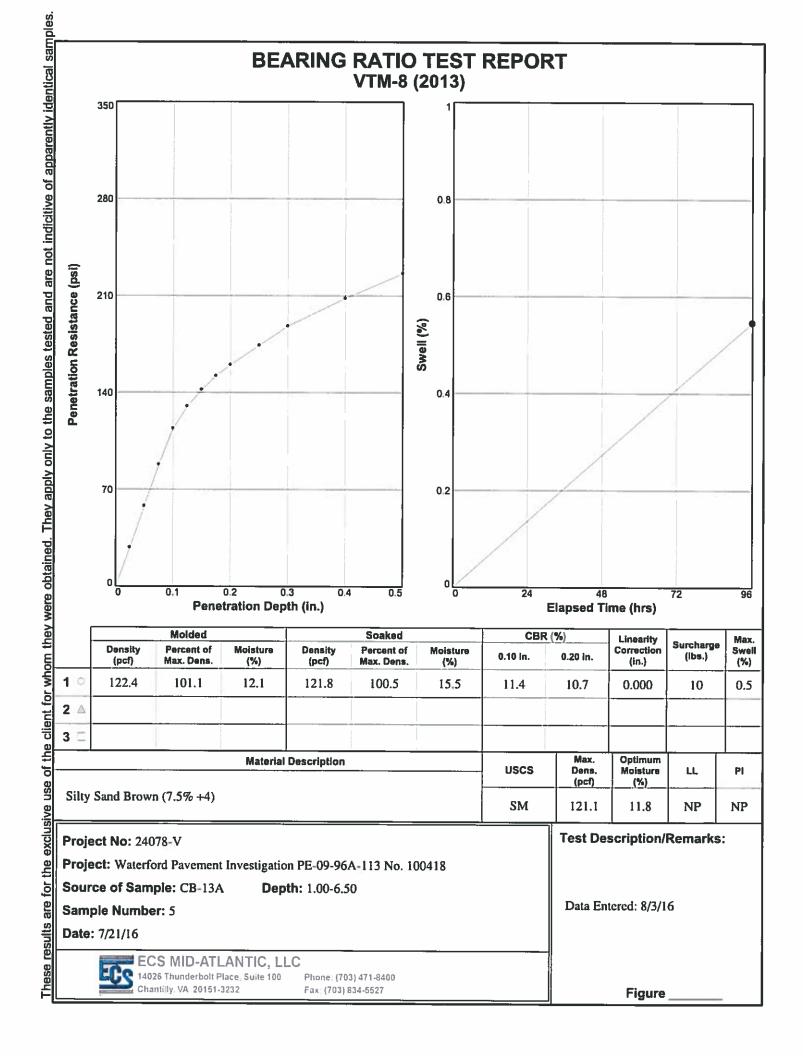
Client: VDOT

Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

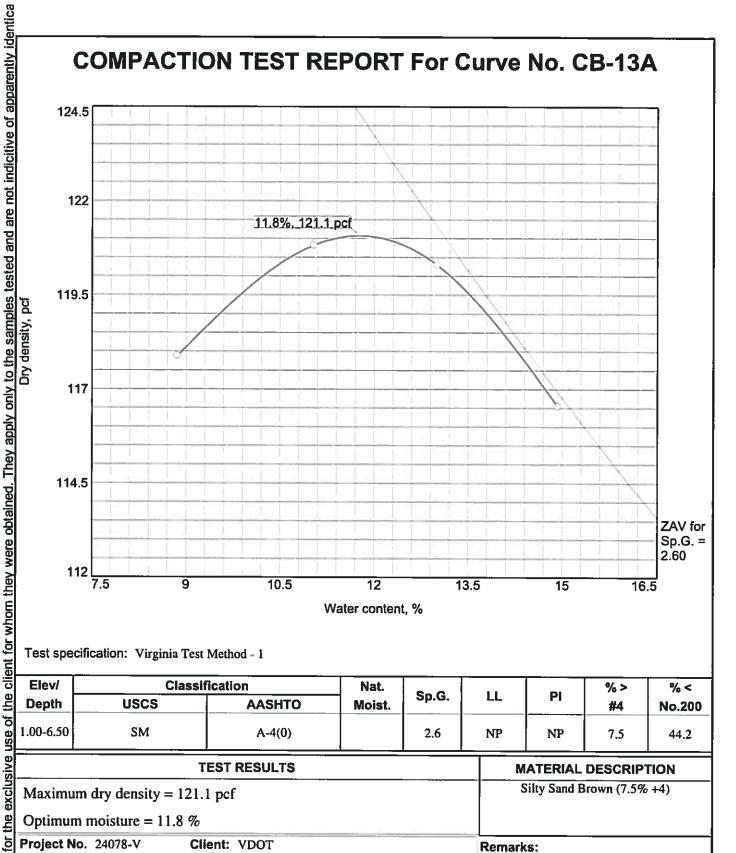
Project No: 24078-V

Figure

Tested By: KV



COMPACTION TEST REPORT For Curve No. CB-13A



Test specification: Virginia Test Method - 1

Elev/	Classif	ication	Nat.	S:: 0			% >	% <
Depth	USCS	AASHTO	Moist.	Sp.G.	L.L.	PI	#4	No.200
1.00-6.50	SM	A-4(0)		2.6	NP	NP	7.5	44.2

Depth	USCS	AASHTO	Moist.	Sp.G.	L.L.	PI	#4	No.200
1.00-6.50	SM	A-4(0)		2.6	NP	NP	7.5	44.2
		TEST RESULTS			M	IATERIAL	DESCRIP	TION

Maximum dry density = 121.1 pcf

Optimum moisture = 11.8 %

Project No. 24078-V

Client: VDOT

Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

Date: 7/23/16

Source of Sample: CB-13A

Sample Number: 5

ECS MID-ATLANTIC, LLC 14026 Thunderbolt Place, Suite 100

Phone (703) 471-8400 Chantilly, VA 20151-3232 Fax: (703) 834-5527

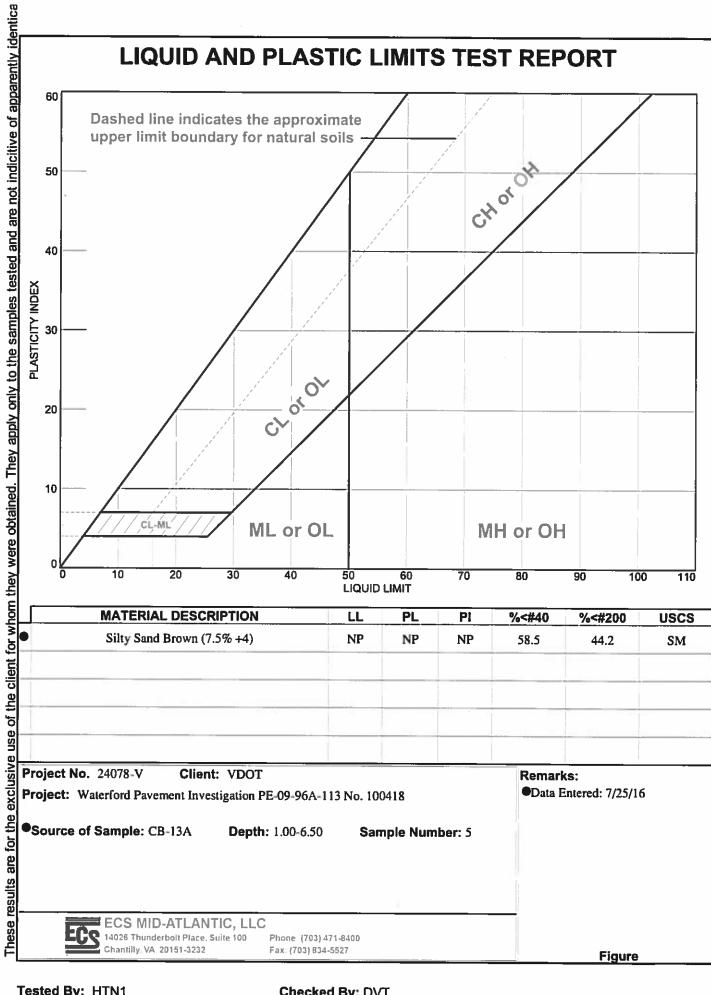
Remarks:

Data Entered: 7/25/16

Figure

Silty Sand Brown (7.5% +4)

Tested By: KV



Tested By: HTN1 Checked By: DVT

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
0.75	100.0		
0.375	97.6		
#4	92.5		
#10	79.7		
#40	58.5		
#60	53.3		
#80	50.7		
#100	49.5		
#200	44.2		
i			

	Soil Description	
Silty Sand Brow	n (7.5% +4)	
PL= NP	Atterberg Limits LL= NP	PI= NP
D ₉₀ = 3.8949 D ₅₀ = 0.1612 D ₁₀ =	Coefficients D ₈₅ = 2.7813 D ₃₀ = C _u =	D ₆₀ = 0.4845 D ₁₅ = C _c =
USCS= SM	Classification AASHT	O= A-4(0)
Data Entered: 7/	Remarks 25/16	

(no specification provided)

Source of Sample: CB-13A Sample Number: 5

Depth: 1.00-6.50

Date: 7/21/16



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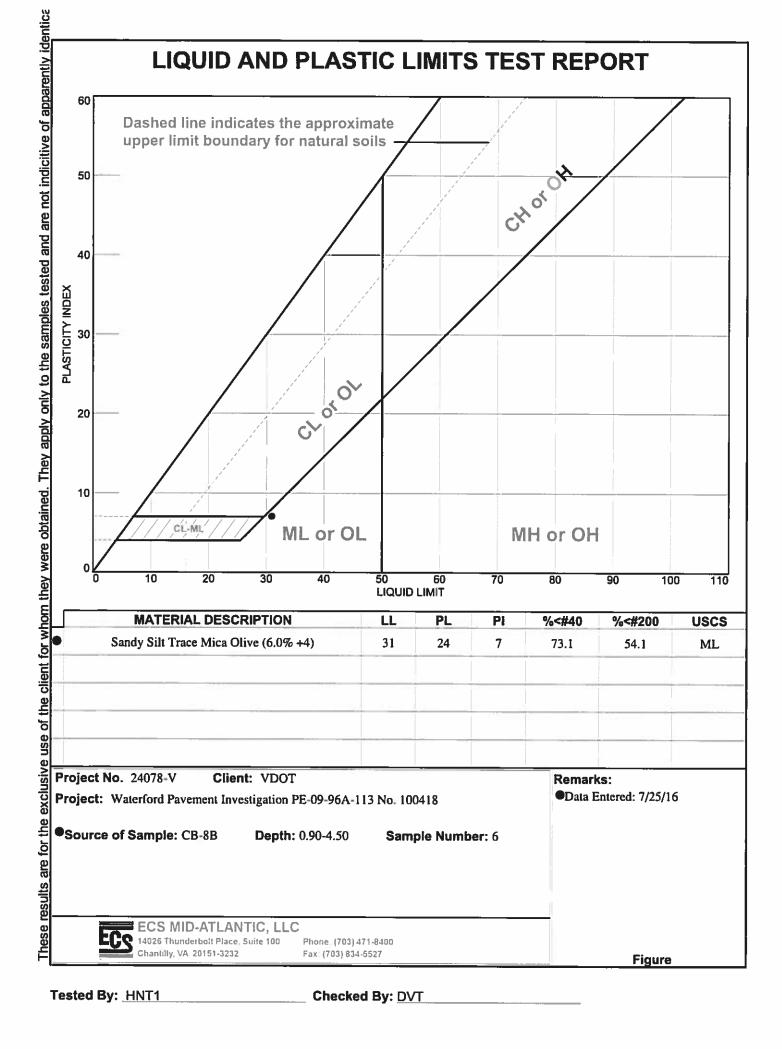
Client: VDOT

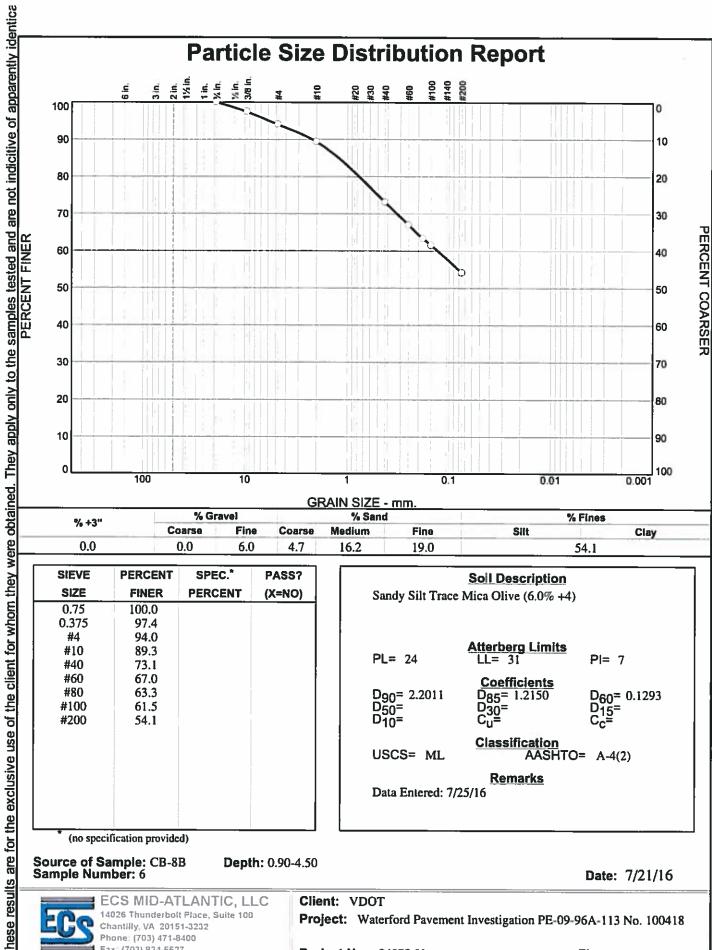
Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

Project No: 24078-V

Figure

Tested By: KV





SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
0.75	100.0		
0.375	97.4		
#4	94.0		
#10	89.3		
#40	73.1		
#60	67.0		
#80	63.3		
#100	61.5		
#200	54.1		
(no spec	ification provide	d)	

	Soil Description	
Sandy Silt Trace	Mica Olive (6.0% +4	()
PL= 24	Atterberg Limits LL= 31	PI= 7
D ₉₀ = 2.2011 D ₅₀ = D ₁₀ =	Coefficients D ₈₅ = 1.2150 D ₃₀ = C _u =	D ₆₀ = 0.1293 D ₁₅ = C _c =
USCS= ML	Classification AASHT	O= A-4(2)
Data Entered: 7/2	Remarks 25/16	

Source of Sample: CB-8B Sample Number: 6

Depth: 0.90-4.50

Date: 7/21/16



ECS MID-ATLANTIC, LLC

14026 Thunderbolt Place, Suite 100 Chantilly, VA 20151-3232 Phone: (703) 471-8400

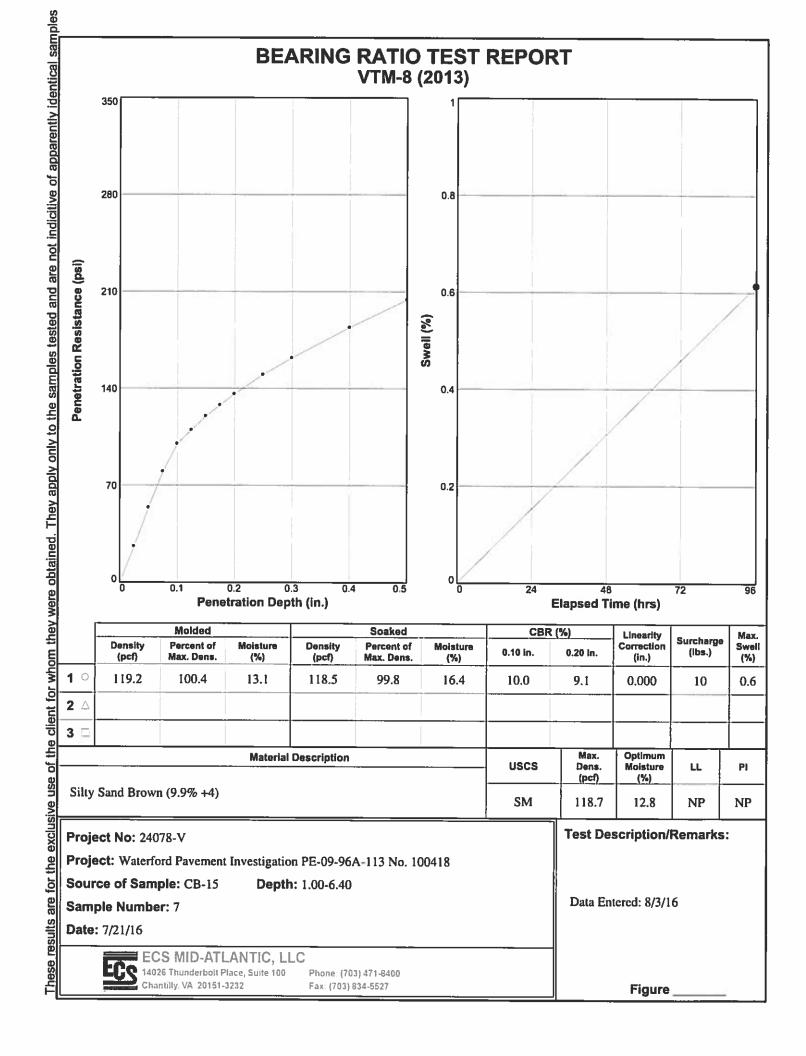
Client: VDOT

Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

Project No: 24078-V

Figure

Tested By: KV



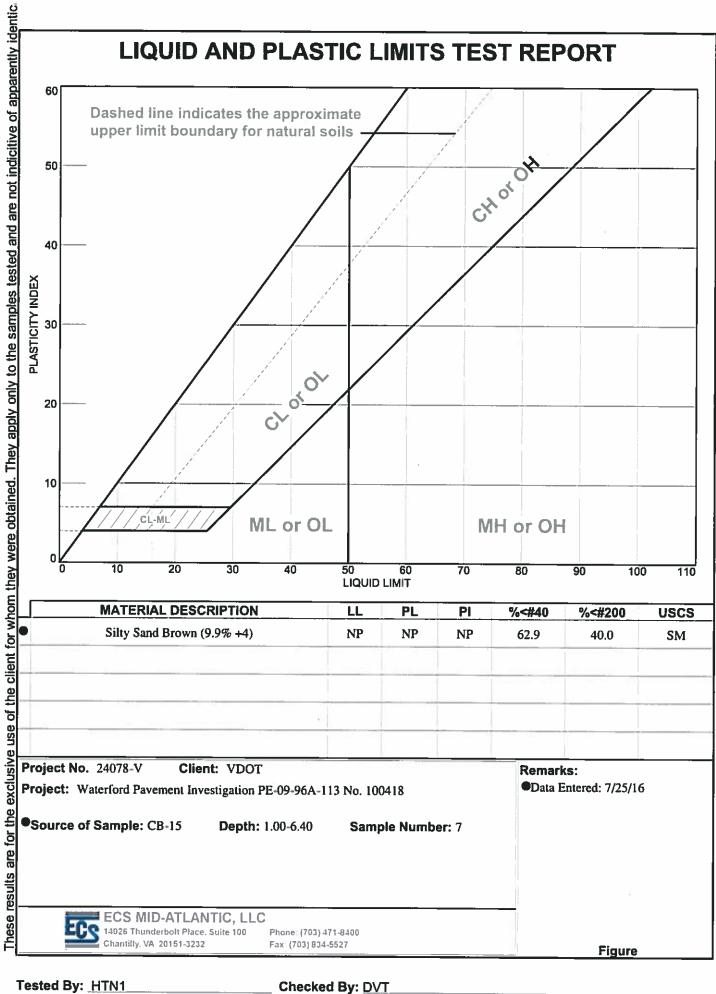
COMPACTION TEST REPORT For Curve No. CB-15 125 120 12.8%, 118.7 pcf 115 110 ZAV for Sp.G. = 2.58 105 100 8 10 12 14 16 18 Water content, %

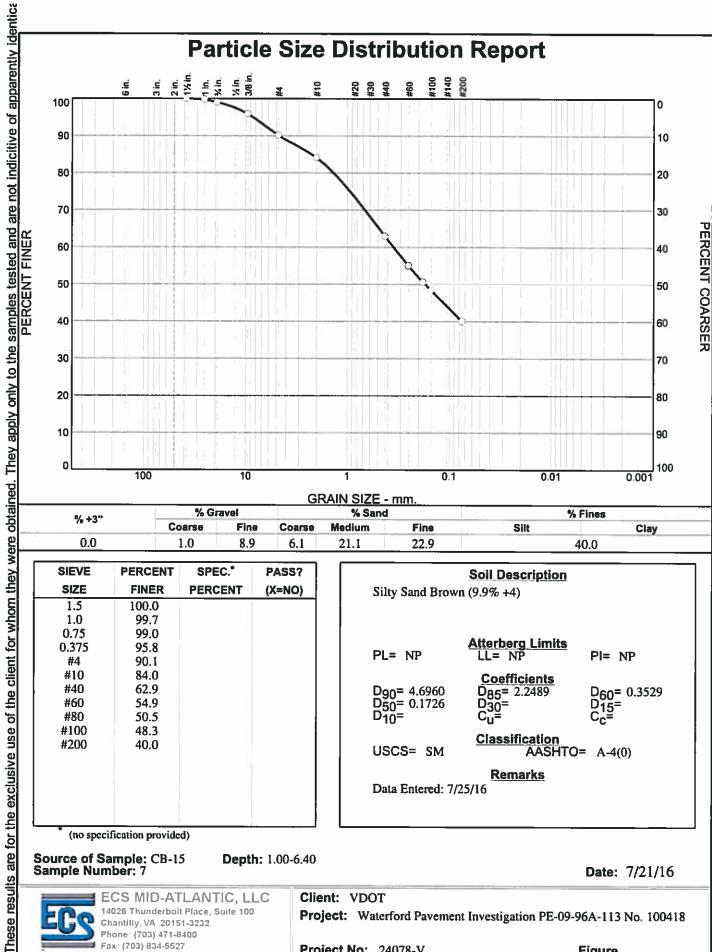
Project No	o. 24078-V	CI	lient: V		-96A-113	3 No. 100418	3 e: 7/22/16	Remari Data En	(s: itered: 7/25/	/16	
	m dry den n moisture	sity = 118 .	_						Silty Sand E	3rown (9.99	% +4)
		<u> </u>	TEST RI						<u> </u>	DESCRIF	
1.00-6.40		M		A-4(0		WOIST.	2.58	NP	NP	9.9	No.2
Elev/ Depth			ification		<u> </u>	Nat. Moist.	Sp.G.	LL	PI	% >	% ·
Test sner	cification: N	Virginia Tesi	t Mathad	- 1	Wa	iter content,					
100	6	8		10		12	14	1	16		18
105											
											2.58
110										55-12-1	ZAV Sp.(
ory density, pa			/								
115	of Park			1	1						
						/ / /					
120					12.89	6 <u>, 118.7 pc</u> f					_
125											

	
TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 118.7 pcf	Silty Sand Brown (9.9% +4)
Optimum moisture = 12.8 %	

Figure

Tested By: KV





PERCENT	SPEC.*	PASS?
FINER	PERCENT	(X=NO)
100.0		
99.7		
99.0		
95.8		
90.1		
84.0		
62.9		
54.9		
50.5		
48.3		
40.0		
1		
	99.7 99.0 95.8 90.1 84.0 62.9 54.9 50.5 48.3	FINER PERCENT 100.0 99.7 99.0 95.8 90.1 84.0 62.9 54.9 50.5 48.3

Coarse

1.0

Fine

8.9

Coarse

6.1

Medium

21.1

Fine

22.9

Silty Sand Brow	Soil Description Silty Sand Brown (9.9% +4)				
PL= NP	Atterberg Limits	PI= NP			
D ₉₀ = 4.6960 D ₅₀ = 0.1726 D ₁₀ =	Coefficients D ₈₅ = 2.2489 D ₃₀ = C _u =	D ₆₀ = 0.3529 D ₁₅ = C _c =			
USCS= SM	Classification AASH1	ΓO= A-4(0)			
Data Entered: 7/2	Remarks Data Entered: 7/25/16				

Silt

Source of Sample: CB-15 Sample Number: 7

(no specification provided)

0.0

Depth: 1.00-6.40

Date: 7/21/16

Clay

40.0

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Client: VDOT

Project: Waterford Pavement Investigation PE-09-96A-113 No. 100418

Project No: 24078-V

Figure

Tested By: KV